LAND INFORMATION MEMORANDUM NO: LM2301474 Received: 14 Nov 2023 Issued: 20 Nov 2023 Section 44A, Local Government Official Information And Meetings Act 1987

APPLICANT

G Hessell, J Davis C/- Meyer Real Estate PO Box 409126 Snells Beach 0942

SITE INFORMATION

Property ID: 117290

Street Address: 1830 Ngunguru Road Whangarei 0173

Legal Description: LOT 2 DP 361651

This is a Land Information Memorandum only.

Full payment has been made for this Land Information Memorandum.



1: PROPERTY DETAILS.

Location Map

Deposited Plan: DP 361651

Record of Title: 250180

This property is subject to a Consent Notice, information attached.

Consent Notice dated 20/01/2006 – Interest Number 6808957.3

2: INFORMATION IDENTIFYING EACH (IF ANY) SPECIAL FEATURE OR CHARACTERISTIC OF THE LAND CONCERNED, INCLUDING BUT NOT LIMITED TO POTENTIAL EROSION, AVULSION, FALLING DEBRIS, SUBSIDENCE, SLIPPAGE, ALLUVION, OR INUNDATION, OR LIKELY PRESENCE OF HAZARDOUS CONTAMINANTS, BEING A FEATURE OR CHARACTERISTIC THAT IS KNOWN TO THE WHANGAREI DISTRICT COUNCIL.

Whangarei District Council holds indicative information on land stability hazard for Whangarei. Information on land stability, including an interactive web tool, can be found on the WDC website. The Whangarei District Council may require site-specific investigations before granting future subdivision or building consent for the property, the level of investigation or assessment would depend on the level of stability risk of the area the property is in.

See map attached indicating this property is located within low, moderate, and high zones and refer:

https://www.wdc.govt.nz/Services/My-property-and-rates/Natural-hazards

This property is in an area identified as a Flood Susceptible Area. See map attached and refer:

https://www.wdc.govt.nz/Services/My-property-and-rates/Natural-hazards

Whangarei District Council notified Plan Change 1 - Natural Hazards (PC1) on the 31st of May 2023. The Plan Change proposes to replace the existing Natural Hazards chapter in the District Plan Operative in Part 2022 with a new Natural Hazards chapter and new rules for subdivision and land use in hazard prone areas. Refer to map attached and for more information on the proposed plan change please visit: https://www.wdc.govt.nz/Services/Planning/District-Plan-changes/Current-plan-changes

This property is in an area that has been identified to contain:

Acid Sulphate Soil Risk

A copy of the Opus Acid Sulphate Soil guidance document dated August 2015 can be found on the Whangarei District Council website.

For information refer:

 $\underline{\text{https://www.wdc.govt.nz/Council/Council-documents/Policies/Acid-Sulphate-Soil-Planning-Policy}}$



Whangarei District Council holds information on the liquefaction vulnerability of the district. The site is located within an area classified as Liquefaction vulnerability category undetermined.

The report was prepared by Tonkin & Taylor Ltd to provide WDC with a district wide liquefaction vulnerability assessment to help inform spatial planning and assessment of landuse, subdivision and building consents.

To view the report and access maps please use the following link: https://www.wdc.govt.nz/Services/My-property-and-rates/Natural-hazards

Please note: To view the liquefaction layer your map scale must be greater than 1:5000.

3: INFORMATION ON COUNCIL AND PRIVATE UTILITY (SEWERAGE, WATER & STORMWATER) SERVICES.

Information relating to Council Utility Services for this property is attached.

Pipeline Assets Map

As-Built, House Connection and/or Drainage Plan for this property from the building file is attached.

As-Built Services Plan from file BC1700484

Pursuant to Section 51 of the Building Act 2004 and Section 451 of the Local Government Act 1974, any future building work that encroaches upon any Council Pipe or Utility must obtain written consent from the Waste & Drainage and/or Water Services Manager/s prior to works commencing.

For information refer: https://www.wdc.govt.nz/Council/Council-documents/Policies/Building-Over-Public-Sewers-Policy

4: INFORMATION RELATING TO VALUATION, LAND, AND WATER RATES. INFORMATION FROM WHANGAREI DISTRICT COUNCIL RECORDS.

Information on Valuation and Rates for the current financial year, is attached.

5: INFORMATION CONCERNING ANY PERMIT, CONSENT, CERTIFICATE, NOTICE ORDER, OR REQUISITION AFFECTING THE LAND OR ANY BUILDING ON THE LAND PREVIOUSLY ISSUED BY THE WHANGAREI DISTRICT COUNCIL OR BUILDING CERTIFIER (WHETHER UNDER THE BUILDING ACT 1991 AND/OR 2004 OR ANY OTHER ACT).

Copy of Building Consents and Code Compliance Certificates issued for this property are attached.

BC1100842 – 2 New Retaining Walls
 Building Consent – Issued 19/10/2011
 Code Compliance Certificate – Issued 02/04/2012



 BC1700484 – New Dwelling Building Consent – Issued 10/07/2017 Code Compliance Certificate – Issued 12/03/2018

6: INFORMATION RELATING TO THE USE TO WHICH THE LAND MAY BE PUT AND ANY CONDITIONS ATTACHED TO THAT USE.

This property is located in a Settlement Zone Residential Sub-Zone. See map attached and refer to Part 3: Area Specific Matters - Chapters - Rural zones. https://www.wdc.govt.nz/Services/Property/Planning/Operative-District-Plan

This property is located in a Coastal Environment.

See map attached and refer to Part 2: District Wide Matters - General District Wide Matters - Coastal Environment

https://www.wdc.govt.nz/Services/Property/Planning/Operative-District-Plan

7: INFORMATION WHICH IN TERMS OF ANY OTHER ACT HAS BEEN NOTIFIED TO THE WHANGAREI DISTRICT COUNCIL BY ANY STATUTORY ORGANISATION HAVING THE POWER TO CLASSIFY LAND OR BUILDINGS FOR ANY PURPOSE.

Whangarei District Council is not aware of any classification attached to the land or building/s.

8: OTHER INFORMATION CONCERNING THE LAND AS WHANGAREI DISTRICT COUNCIL CONSIDERS, AT COUNCILS DISCRETION, TO BE RELEVANT.

Whangarei District Council recommends that all Whangarei District residents visit the Northland Regional Council website, https://www.nrc.govt.nz/ for information on Civil Defence hazard response. This information includes Tsunami evacuation zones, maps and community response plans for flooding and extreme weather events etc.

Copies of site plan, floor plan and elevations are attached for your information.

A copy of Cook Costello "Design Summary – Retaining Walls" dated 6/03/2011 from file BC1100842, is attached for your information

A copy of Cook Costello "Geotechnical Report" dated 03/10/2016 from file BC1700484, is attached for your information

9: INFORMATION RELATING TO ANY UTILITY SERVICE OTHER THAN COUNCILS SUCH AS TELEPHONE, ELECTRICITY, GAS AND REGIONAL COUNCIL WILL NEED TO BE OBTAINED FROM THE RELEVANT UTILITY OPERATOR.

Further information may be available from other authorities; Northpower; Spark; Vector Limited; etc.



DISCLAIMER

Land Information Memoranda (LIM) are prepared under the provisions of Section 44A of the Local Government Official Information and Meetings Act 1987. An inspection of the land or building(s) has not been completed for the purposes of preparing the LIM. It has been compiled from the records held by Whangarei District Council. The information contained in the LIM is correct at the date of issue.

A LIM is prepared for the use of the applicant and may not be able to be relied on by other parties.

Advice from an independent professional such as a lawyer or property advisor should be sought regarding the contents of this LIM.

Additional information regarding the land or buildings (such as resource consents and other permissions and restrictions) not contained in this LIM may be held by Northland Regional Council. For further information contact Northland Regional Council on (09) 470 1200, 0800 002 004 or www.nrc.govt.nz.

A LIM is not a suitable search of Council's records for the purposes of the National Environmental Standards (NES) for soil contamination of a potentially contaminated site.

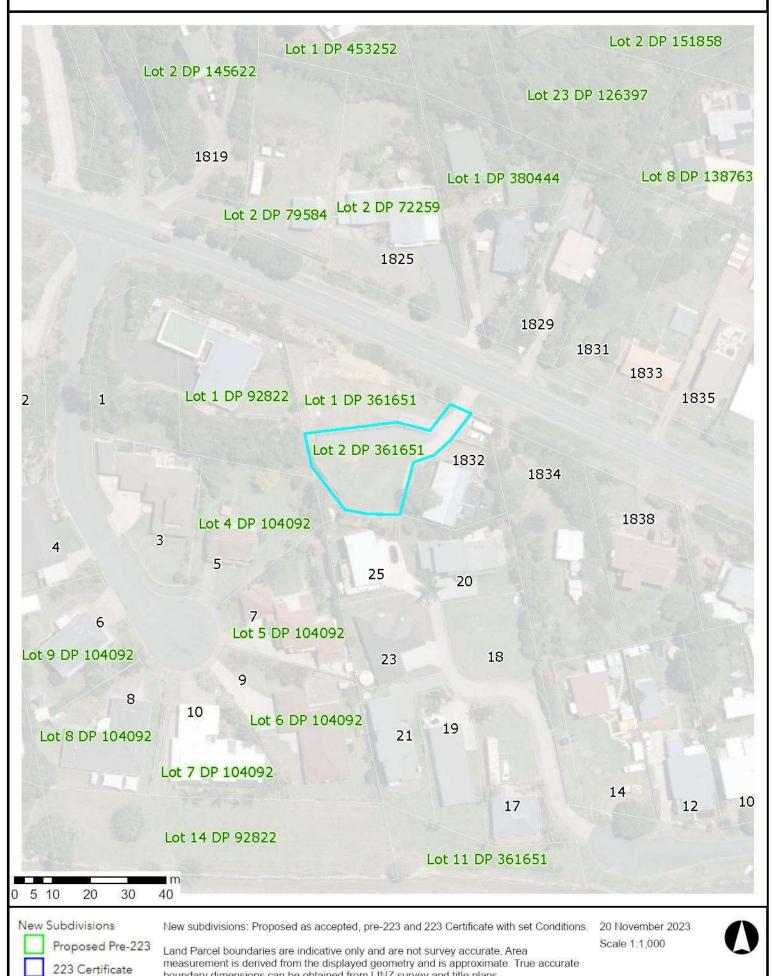
Signed for and on behalf of Council:

W Copeland

Property Assessment Officer

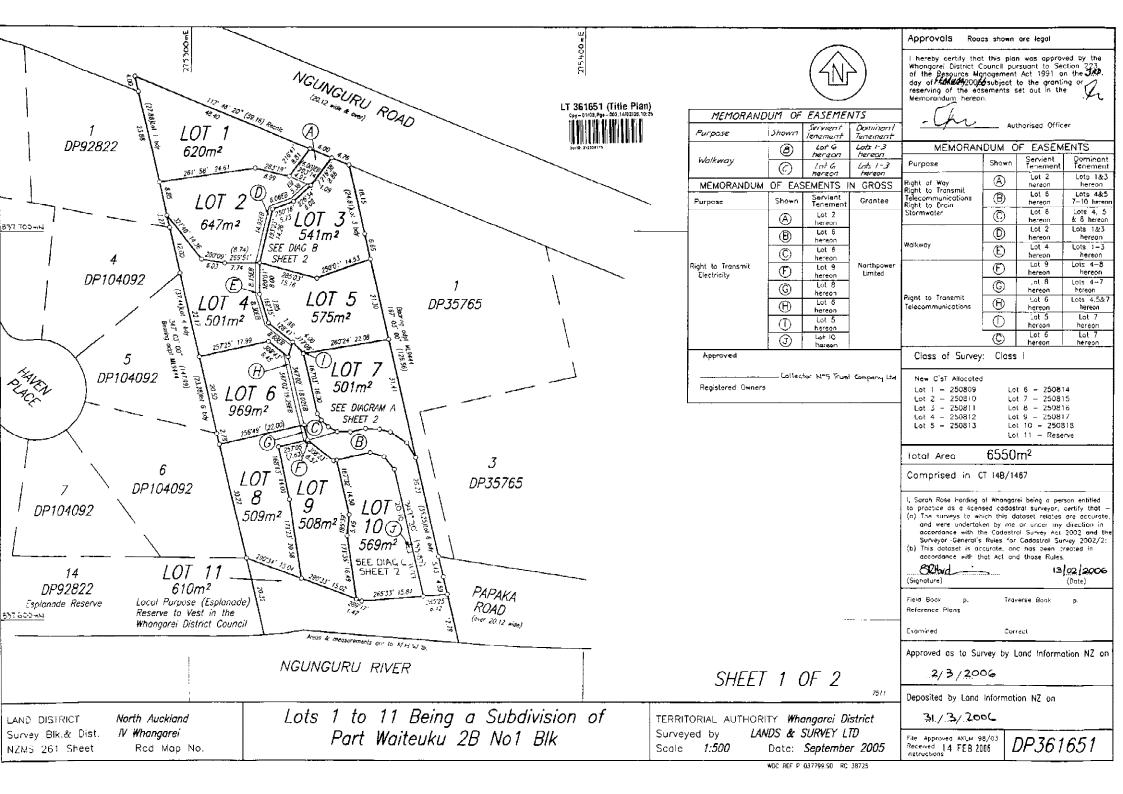
Property Map

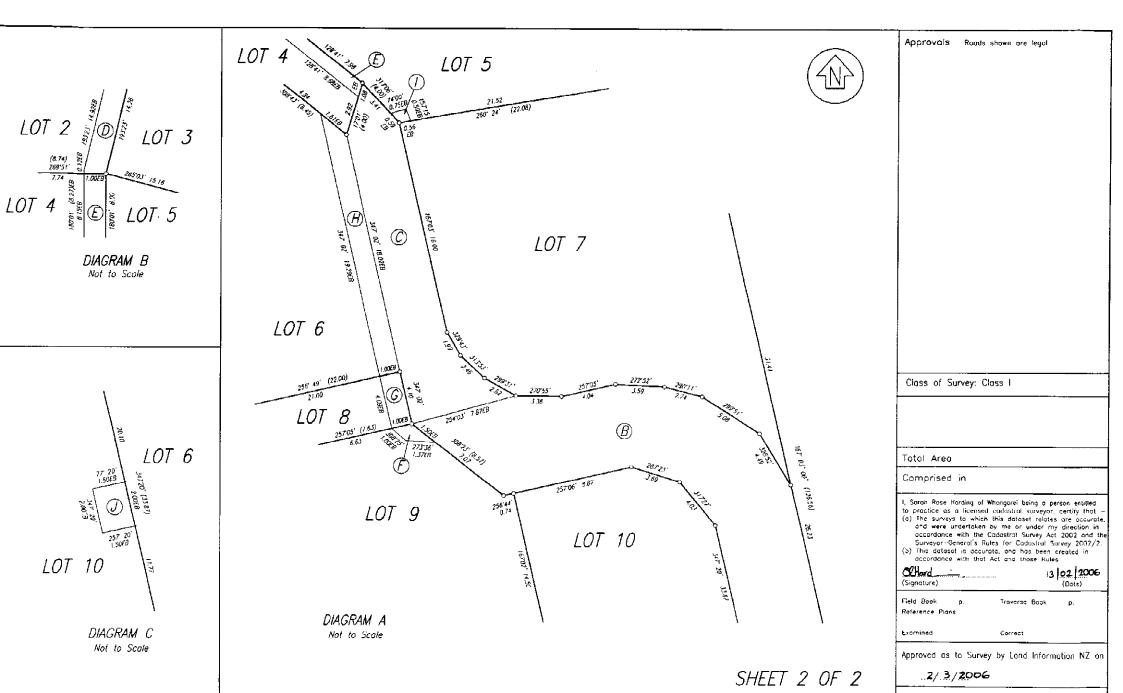




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boundary dimensions can be obtained from LINZ survey and title plans





LAND DISTRICT Survey Blk.& Dist. NZMS 261 Sheet

North Auckland IV Whangarei Red Map No. Lots 1 to 11 Being a Subdivision of Part Waiteuku 2B No1 Blk

TERRITORIAL AUTHORITY Whangarei District
Surveyed by LANDS & SURVEY LTD
Scale As Shown Date: September 2005

Deposited by Land Information NZ on

31/3/2006

File Approved AKLM 99/03 Received 1 FEB 2006 DP361651



RECORD OF TITLE UNDER LAND TRANSFER ACT 2017 FREEHOLD





Identifier 250810

Land Registration District North Auckland

Date Issued 31 March 2006

Prior References NA14B/1467

Estate Fee Simple

Area 647 square metres more or less
Legal Description Lot 2 Deposited Plan 361651

Registered Owners

Guy Robert Hessell as to a 1/2 share Jenny Maree Davis as to a 1/2 share

Interests

6808957.3 Consent Notice pursuant to Section 221 Resource Management Act 1991 - 31.3.2006 at 9:00 am Subject to a right of way, rights to transmit telecommunications and drain stormwater easements over part marked A and walkway easement over part marked D on DP 361651created by Easement Instrument 6808957.5 - 31.3.2006 at 9:00 am Appurtenant hereto is a right to walkway easement created by Easement Instrument 6808957.5 - 31.3.2006 at 9:00 am The easements created by Easement Instrument 6808957.5 are subject to Section 243 (a) Resource Management Act 1991 Subject to a right (in gross) to convey electricity easement over part marked A on DP 361651 in favour of Northpower Limited created by Easement Instrument 6808957.6 - 31.3.2006 at 9:00 am

The easements created by Easement Instrument 6808957.6 are subject to Section 243 (a) Resource Management Act 1991 Land Covenant in Easement Instrument 6808957.7 - 31.3.2006 at 9:00 am

11947057.3 Mortgage to Westpac New Zealand Limited - 11.12.2020 at 2:35 pm

CONO 6808957.3 Cons Cpy - 01/01, Pgs - 017, 30/03/06, 10:32 IN THE MATTER

of the Resource Management Act 1991 ("the Act")

AND

IN THE MATTER

of a subdivision consent as evidenced by Land Transfer Plan

No. 361651

<u>A N D</u>

IN THE MATTER

of a Consent Notice issued pursuant to Section 221 of the Act by <u>THE WHANGAREI DISTRICT COUNCIL</u> ("the Council")

IT IS HEREBY CERTIFIED that the following conditions to be complied with on a continuing basis by the subdividing owner and subsequent owners were imposed by the Council as conditions of approval for the subdivision as effected by Land Transfer Plan No. 361651 ("the plan")

- 1. Vehicle access to lots 1 and 3 on the plan shall only be obtained from the right of way shown marked on the plan with the letter "A".
- 2. Any development on the properties being lots 1 through 11 on the plan shall comply with the recommendations and restrictions specified in the engineering report entitled "Ngunguru Motor Camp Stormwater Management" compiled by Richardson Stevens Consultants (1996) Limited dated 7 June 2005, a copy of which is attached hereto, and in particular shall:
 - (i) Direct all stormwater from individual lots to on-site soakage pits with overflow outlets from lots 1, 2 and 3 on the plan and rights-of-way marked "A", "B" and "C" on the plan.
 - (ii) Collect water from roofs for drinking with any overflow to be either discharged into each lots own soakage pit or bypass the infiltration pits.

(3) COMO MAIAM / MO)

- (iii) Direct stormwater from rights-of-way to a soakage pit beneath the pavement.
- (iv) Design on-site disposal pits for lots 1, 2 and 4 through 10 on the plan at the time that building consent applications for buildings on these properties are lodged for consideration.

DATED at Whangarei this MM day of

2006

THE WHANGAREI **SIGNED** for COUNCIL pursuant to the authority of the Council given pursuant to the Local Government Act 2002 and the Resource Management Act 1991

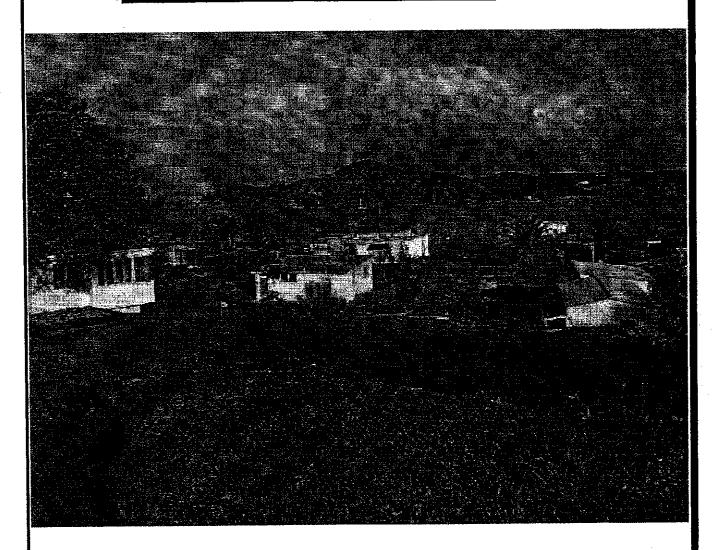
Authorised Signatory

Extract Copy

ENGINEERING REPORT

For

E ENGUNYGURUNNOTOROANIP ZE E SEORNAVASERAVANAGENIEN



RICHARDSON STEVENS CONSULTANTS (1996) LTD.

Date:

07/06/05

File Number:

6242

Report By:

GUY WOOD

Table Of Contents

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Location and Site Conditions	4.
Subsoil Investigations	4.
Percolation Testing	5.
System Design and Calculations	6.
Site Plan	11.
Stormwater Soakage Pit Detail	12.



Email: engineers@richardsonstevens.co.nz



Grant Stevens B.E., M.I.P.E.N.Z. (Civil, Structural) Chartered Professional Engineer

Steve Turner
B.E., M.I.P.E.N.Z. (Civil, Structural)
Chartered Professional Engineer

CIVIL & STRUCTURAL ENGINEERS, 2 SEAVIEW RD, WHANGAREL PH: 09 438 3273, FAX: 09 438 5734

Engineering Report for Stormwater Management Lot 16, DP 323830, Ngunguru

Introduction

The client proposes to subdivide Lot 16, DP 323830. It has been requested that Richardson Stevens Consultants design a stormwater management system to comply with condition 1(i) of the Resource Consent condition which requires details of stormwater control, inclusive subdivision reticulation, disposal, connections and associated calculations to be shown on the engineering plans.

Site Description/ Proposed Development

This is a 6589m² site located off Ngunguru Road with another entrance Papaka Road. The site is flat, but split into three levels. The lower level is the largest and contains 8 of the proposed 11 lots. Ground cover is lawn. Some mature trees are to remain on the site.

A right-of-way is to be constructed off Papaka Road to access Lots 4-10 on the lower level. There will also be a right-of-way off Ngunguru Road to access Lots 1-3 on the upper levels. The subdivision scheme plan prepared by Lands & Survey Ltd is attached.

Site Investigations

Two boreholes and soakage tests were carried out on the site. The bores were dug to a depth of 1.0m by Richardson Stevens Consultants on the 2nd June 05; the soil type is sand on the lower level and low-medium clay overlying greywacke on the upper levels. No groundwater was encountered in the subsoil investigations. Rapid soakage tests were carried out in the boreholes. The result after 7 minutes of testing on the lower level was 580mm and 310mm was recorded on the upper level after 30 minutes. The bore log and soakage tests results are attached in the appendices.

From these tests Richardson Stevens Consultants conclude that the soil type on the lower level of the site is a category 1 soil, sand, rapidly drained and that on the upper slopes is a category 5 soil, light clay, poorly drained (as per NZS 1547:2000).

Stormwater Control

Lots 1-3

These Lots are underlain by clay over weathered greywacke rock. This soil type does not have sufficient infiltration capacity to achieve full on-site disposal of stormwater. Stormwater connections to the roadside drain along Ngunguru Road are therefore required to service these Lots. To provide stormwater treatment however to comply with the requirement of ARC TP10 stormwater from impervious surfaces (excluding roofs), should be directed to the stormwater connections via infiltration pits. The pits will need to have sufficient capacity to handle one-third of the 2-year storm event (ARC TP10 requirement). Our calculations indicate that for an impervious area of 100m² a pit 2.7m x 1.2m x 1.5m deep will be required. Roof water is expected to be collected in storage tanks. The overflow from these tanks should be piped to the stormwater connection downstream of the infiltration pit as water quality from sources used for drinking water can be assumed to be free of contaminants of concern to the environment. The stormwater connections will need to be installed at the subdivision stage however the on-site disposal pits for lots 1 and 2 should left until building consent stage as the actual size and location required cannot be determined until that time.

Lots 4 - 10

The sandy soils on the lower portion of the subdivision (Lots 4-10) are suitable for on-site disposal of stormwater. Calculations appended indicate that a pit size of 1.7m x 1.2m x 1.0m would be required for every 100m² of impervious surface for a 10 year event. Roof water is expected to be collected in storage tanks with overflow to a nominal soakage pit. No stormwater connections are therefore proposed for these Lots. The size and location of the on-site disposal pit for each Lot will need to be included with the building consent applications.

ROW A

This ROW is 40m². Soils are clay over weathered greywacke. A pit size of 1.2m x 1.0m deep is required to comply with the ARC TP10 requirements. This pit will need to be installed under the paved surface due to space limitations. A 150mm thick reinforced concrete pavement is therefore recommended for this ROW to span over the pit. The overflow/outlet from the pit will discharge via a 100mm stormwater connection to the water table drain along Ngunguru Road.

ROW B & C

This ROW has an imperious area of some 400m². Soils are free draining sand. An infiltration pit size of 6.5m x 0.6 wide x 1.0m deep is required to comply with ARC TP10 requirements. As with ROW A the pit will need to be installed under the pavement due to space limitations. The pit is only 600mm wide therefore a 125mm pavement is considered adequate, however an additional layer of 665 mesh should be laid over the trench to account for any possible settlement. The overflow/outlet from the pit will discharge via a 225mm diameter culvert to Ngunguru River. An Enviropod or similar filter will be required in the sump to prevent clogging of the soakage pit.

Conclusions

Richardson Stevens Consultants conclude that all stormwater from individual lots should be directed to on-site soakage pits with overflow outlets from Lots 1-3, and ROW A, B & C. This does not include water from roofs as this is to be collected for drinking and any overflow will be either discharged into its own soakage pit or bypass the infiltration pits. Stormwater from the right-of-ways should be directed to a soakage pit beneath the pavement as shown on the site plans included in the appendices. The design is based on a soakage rate of 1000 mm/hr on the lower slopes and 300 mm/hr on the upper slopes.

Note

Recommendations and opinions in this report are based on data obtained as previously detailed. The nature and continuity of subsoil conditions away from the test holes are inferred and it should be appreciated that actual conditions could vary from those assumed.

Prepared by:

Suy Wood

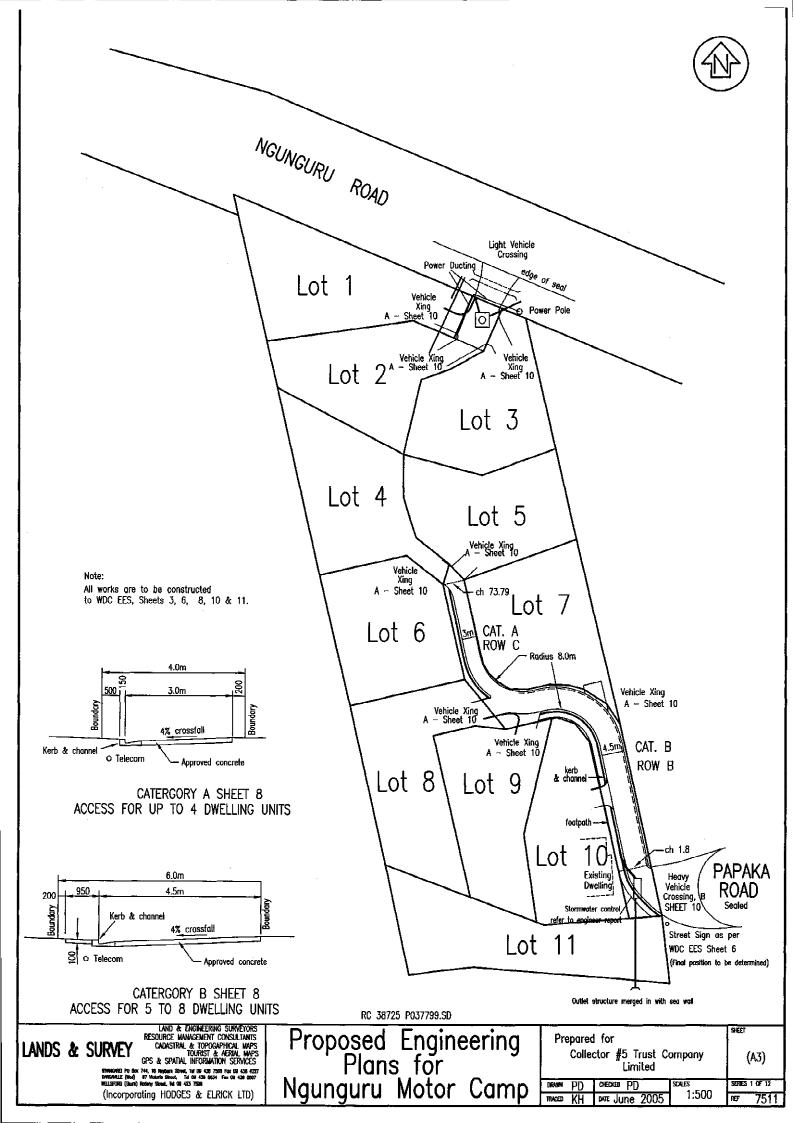
Graduate Engineer

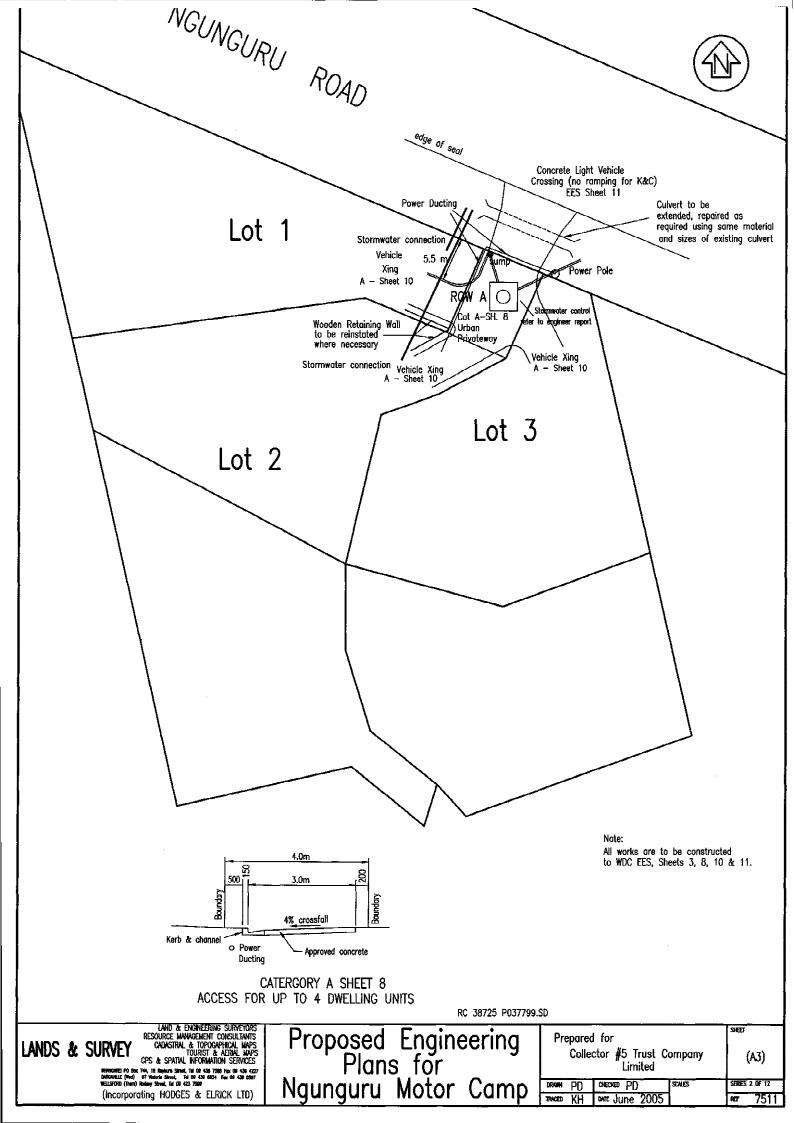
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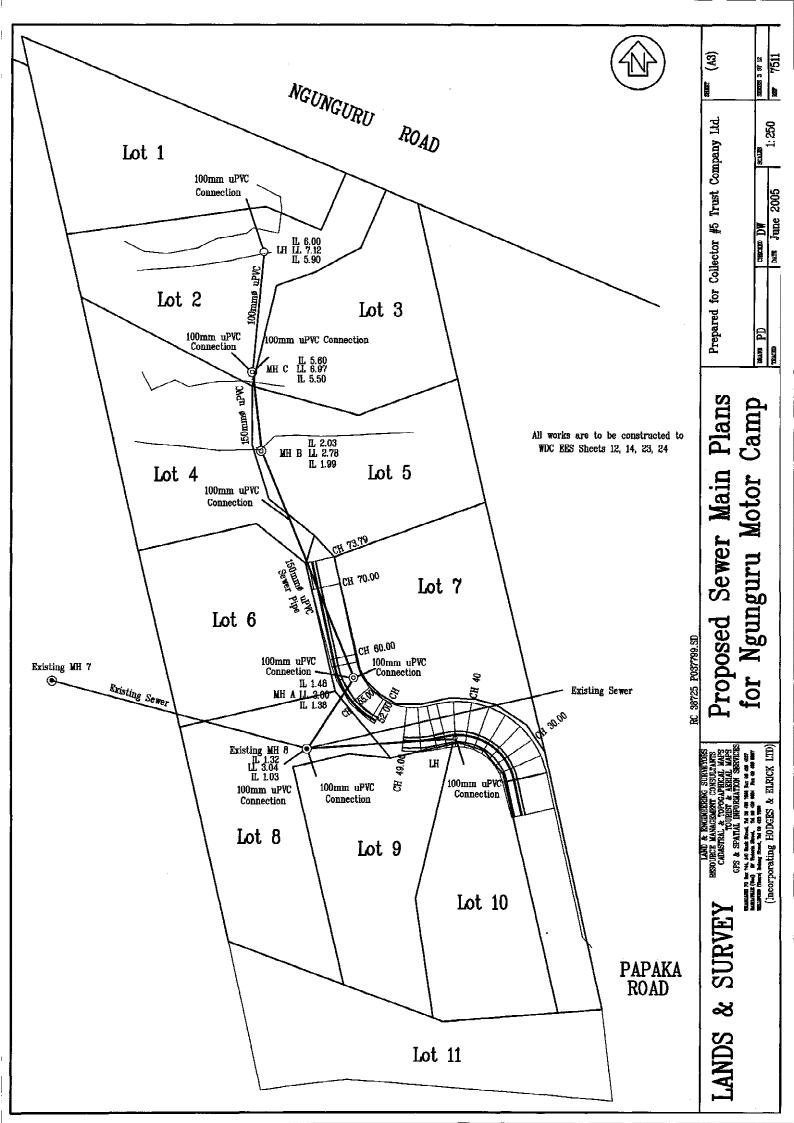
Steve Turner

Chartered Professional Engineer

RICHARDSON STEVENS CONSULTANTS (1996) LTD.

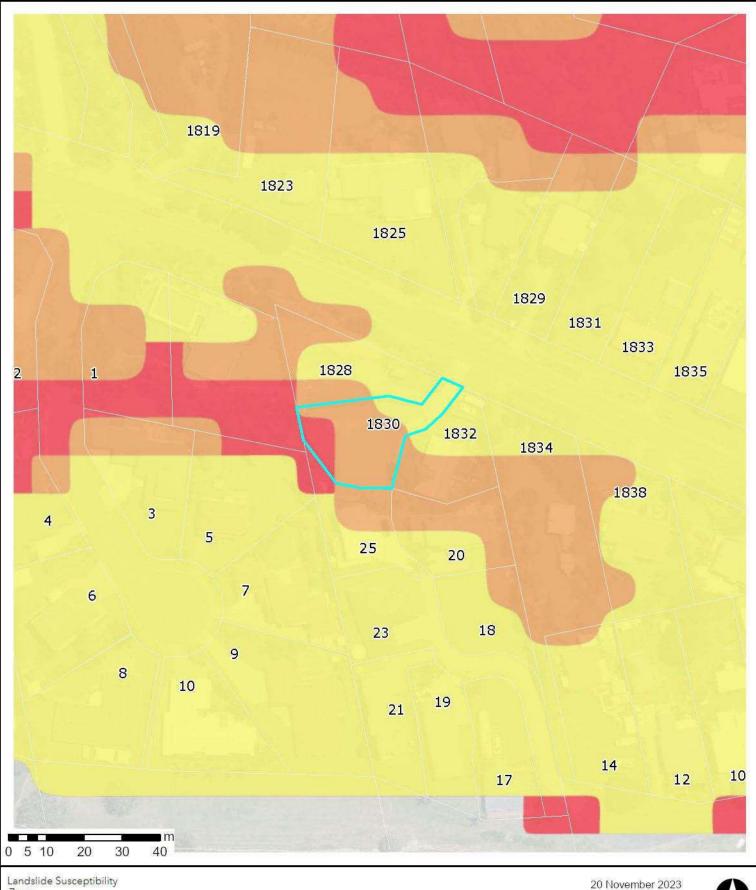






Land Stability





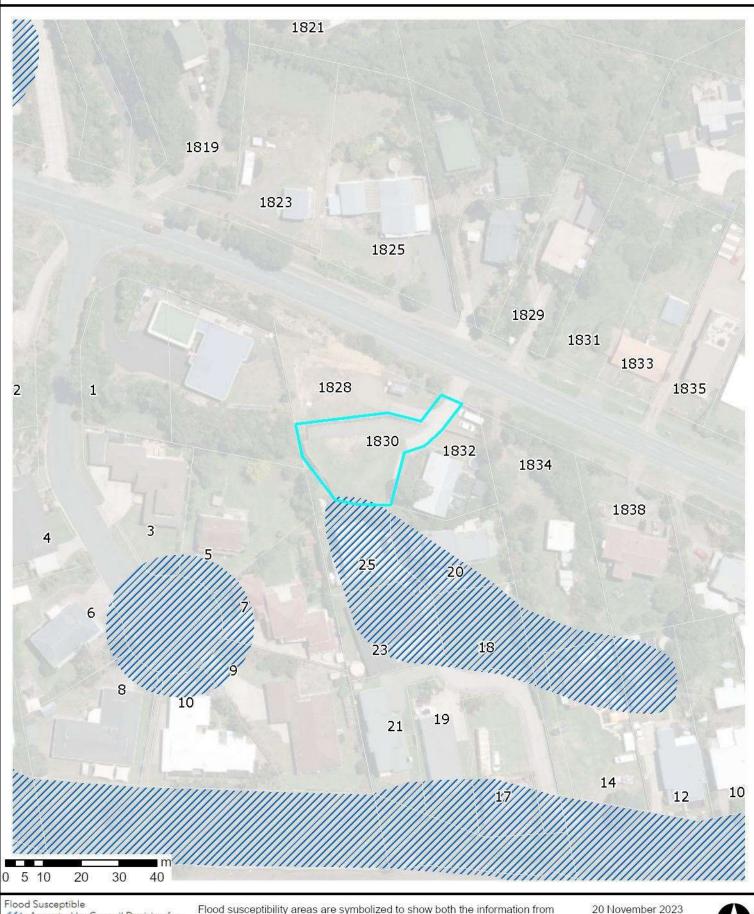
Landslide Susceptibility Zone High The Whangarei District Council may require site-specific investigations before granting future Moderate reports/hazard-reports/land-stability/landslide-susceptibility-technical-report.pdf

Scale 1:1,000 Whangarei District Council holds indicative information on land stability hazard for Whangarei. subdivision or building consent for the property, depending on the level of stability risk of the area the property is in. Tonkin + Taylor Ltd Landslide Susceptibility assessment report: https://www.wdc.govt.nz/files/assets/public/documents/council/

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Flood Susceptibility Review





Flood Susceptible

Accepted by Council Decision for District Plan

Variation not accepted by Council

Flood susceptibility areas are symbolized to show both the information from the District Plan Flood Susceptible Areas: 'Accepted by Council Decision' (in diagonal blue) and Additional Flood Susceptible Areas: 'Yet to be accepted by Council' (in diagonal green) Floods information shown is approximate and should neplacement for site specific investigation and assessments. The absence of bazar

Council' (in diagonal green) Floods information shown is approximate and should not be used as a replacement for site specific investigation and assessments. The absence of hazard information shown does not mean that there is none, only that the information may not yet have been collected.

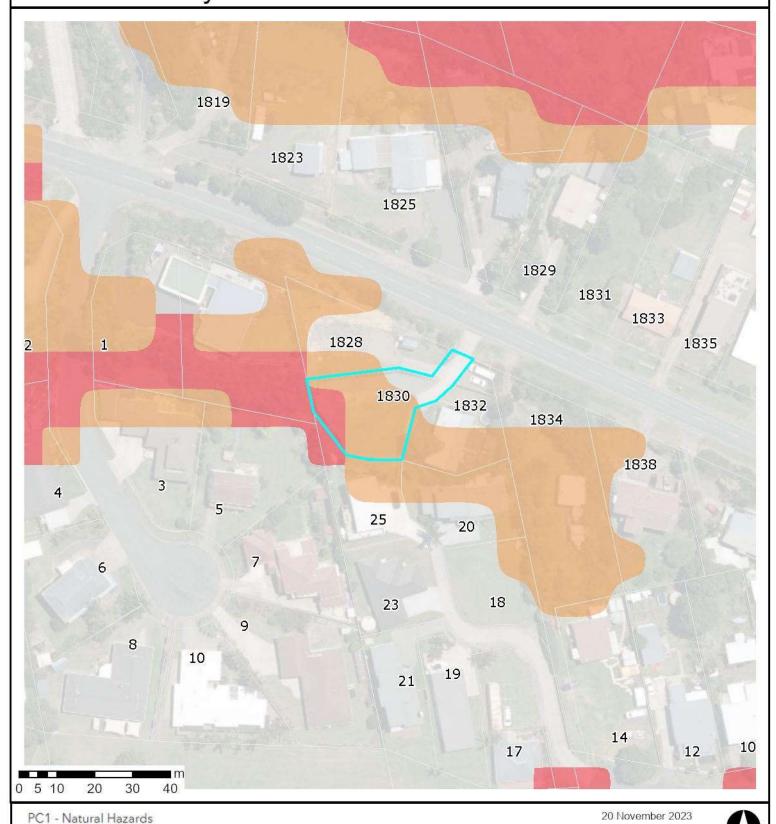
Scale 1:1,000

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District Plan Change 1 - Natural Hazards Land Instability



Scale 1:1,000





Instability

Moderate Susceptibility to Land Instability

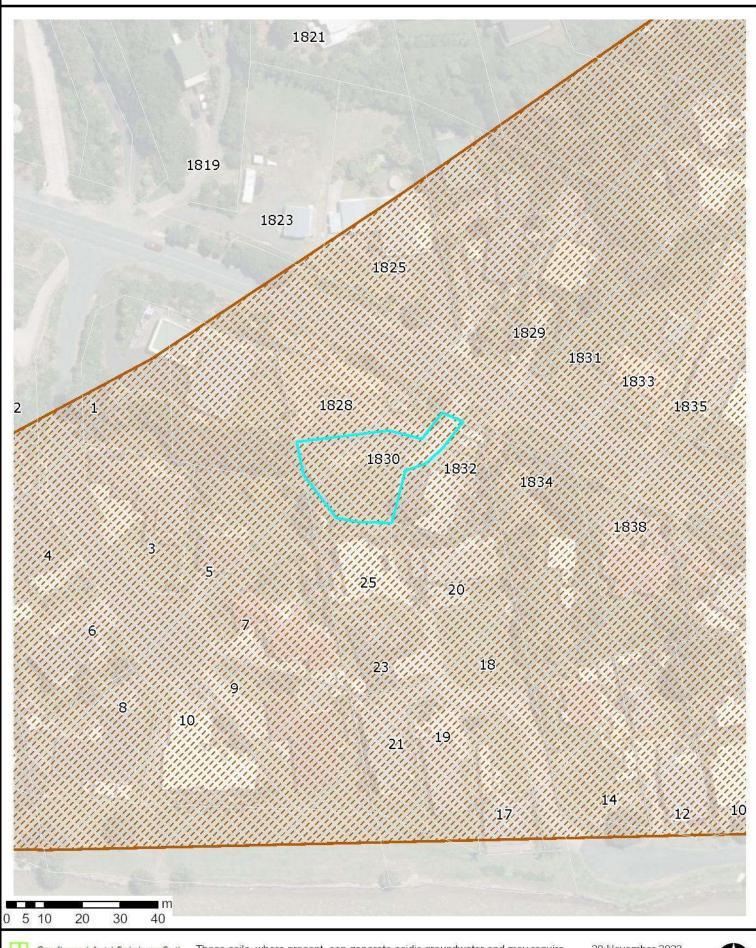
Information provided on this map forms part of Plan Change 1 - Natural Hazards.

To view the proposed maps and see how the changes may affect the property please visit: https://www.wdc.govt.nz/Services/Planning/District-Plan-changes/Current-plan-changes.

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Acid Sulphate Soil (Risk/Confirmed)





Confirmed Acid Sulphate Soil
Acid Sulphate Soil Risk

These soils, where present, can generate acidic groundwater and may require consideration with regard to land drainage and selection of building materials for buried structures.

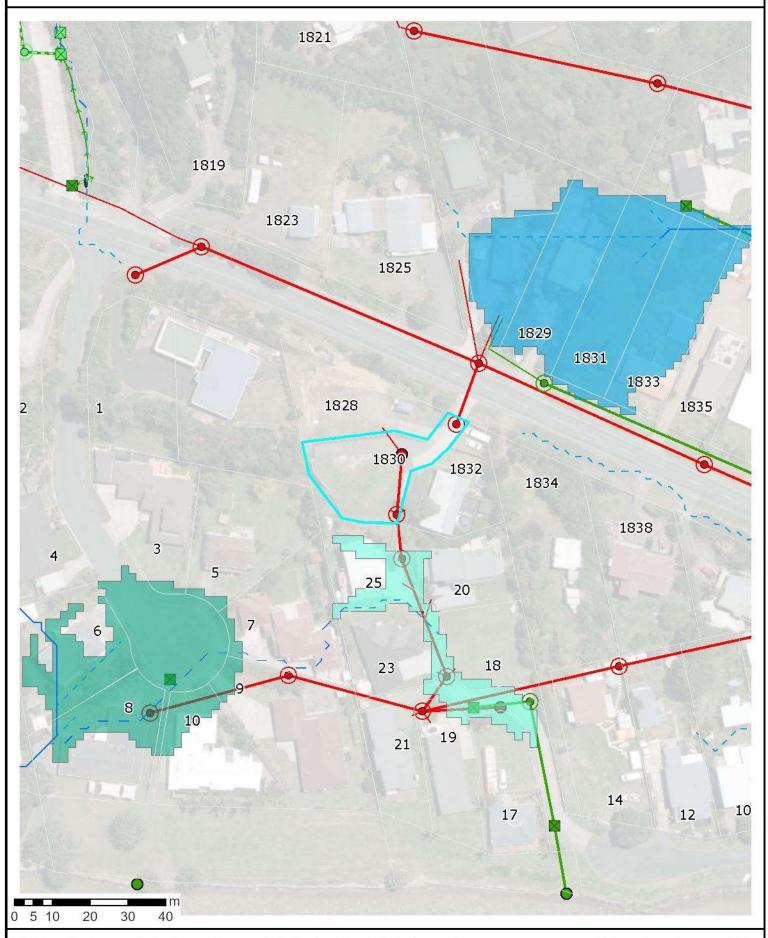
20 November 2023 Scale 1:1,000



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Pipeline Assets





This information is generalized and shows the approximate location of the Public pipeline services. For digging, the As-Built engineering drawings must be used to accurately locate the services. See WDC Customer Services.

20 November 2023 Scale 1:1,000

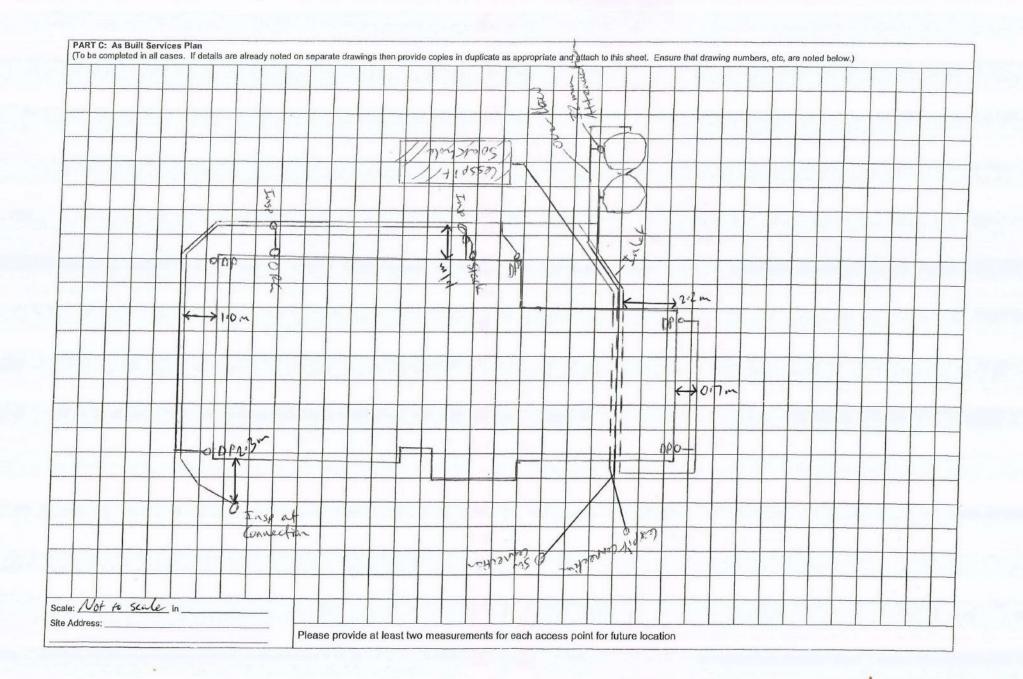


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Pipeline Assets - Map Legend



Stormwater Catchment and Flood Water Stormwater Wastewater Management Water Point Wastewater Point Stormwater Point Actuator Backflow Device Overland Flow Paths 2021 End Structure WDC WDC Modelled Catchment Flowpaths 2021 WDC Private Private - - 0.2 ha to 0.4 ha Private Backflow Device Bore - - 0.4 ha to 1.0 ha Fitting Node WDC ₩DC 1.0 ha to 3.0 ha WDC Private Private Private - 3.0 ha to 100.0 ha Bore ₩DC - 100.0 ha and above End Structure GPT Private WDC ■ WDC Surface Depression Ponding Areas 2021 Private End Structure Private 0.200000 - 0.600000 m WDC Fitting Node Manhole 0.600001 - 1.200000 m Private WDC (WDC Private 1.200001 - 2.000000 m Fitting Node O Private Manhole WDC 2.000001 - 4.000000 m Private (WDC Pump 4.000001 - 9.910000 m Hydrant WDC O Private WDC Meter Private Private Overland Flow Paths 2017 WDC WDC Catchment Area 2017 Stormwater Inlet Private - 0.2 - 1.0 Ha WDC WDC **WDC** Motor Control Centre Meter Manifold Private 1.0 - 2.0 Ha ■ WDC - 2.0 - 5.0 Ha Valve Private Private - > 5.0 Ha ⊗ WDC Pump Pump Private Depression Storage Areas 2017 WDC WDC Depression Storage Areas Private Private Stormwater Line Valve Valve Abandoned Pipe ⊗ WDC ₩DC Private Private Culvert - WDC Water Line Wastewater Line Private Abandoned Pipe Abandoned Pipe Drainage WDC Trunk Main -- Private WDC Rising Main (Pressure) - Private Main Sewer Gravity Main - WDC Other Main Private Private - WDC - Other Process Pipework Private WDC Process Pipework --- Private Process Pipework WDC - WDC --- Private Service Line --- Private WDC Service Line - Private Reticulation - WDC WDC Surface Drain - Private Private → WDC - Private Service Line Wastewater Area Chamber Stormwater Area --- Private WDC Chamber Water Area Private WDC Chamber Pressure Sewer System WDC Private Public Private Private Reservoir WDC Private





Whangarei District Council Private Bag 9023, Te Mai Whangarei 0143 Ph:0-9-430 4200

Email: mailroom@wdc.govt.nz

Rates LIM Report

As at: Friday, 17 November, 2023

Property Number 117290

Legal Description LOT 2 DP 361651 **Assessment Number** 0034030201

Address 1830 Ngunguru Road Whangarei 0173

Record of Title(s) 250810 Land Value \$550,000 Capital Value \$1,225,000 Date of Valuation 01-July-2021

Effective Date (used for rating purposes) 01-July-2022

Meter Location

Rates Breakdown (up to 30 June 2024)

Rates Charge	Charge Total
General Residential	\$1,282.93
Sewage Disposal - Residential	\$902.00
Uniform Annual General Charge	\$701.00
Regional Council Services	\$180.61
Regional Economic Development	\$11.77
Regional Emergency & Hazard Management	\$50.50
Regional Emergency Services Rate	\$11.44
Regional Flood Infrastructure	\$36.56
Regional Land and Fresh Water Management	\$154.11
Regional Pest Management	\$88.06
Regional Sporting Facilities	\$16.37
Regional Transport Rate	\$43.17
Annual Charge Total	\$3,478.52

Opening Balance as at 01/07/2023

\$-0.05

Rates Instalments	Total
20/07/2023 Instalment	\$871.52
20/10/2023 Instalment	\$869.00
20/01/2024 Instalment	\$869.00
20/04/2024 Instalment	\$869.00
Rates Total	\$3.478.52

Balance to Clear \$2,572.39



Building Consent No: BC1100842 Section 51, Building Act 2004

Issued: 19 October 2011
Project Information Memorandum No: PM1100567



Forum North, Private Bag 9023 Whangarei 0148, New Zealand Telephone: +64 9 430 4200 Facsimile: +64 9 438 7632 Email: mailroom@wdc.govt.nz Website: www.wdc.govt.nz

The Building

Street address of building:

1830 Ngunguru Road Whangarei 0173

Legal description of land where building is located:

LOT 2 DP 361651 LLP: 114627

Building name:

N/A

Location of building within site/block number:

N/A

Level/unit number:

N/A

The Owner

M J Day PO Box 278

Whangaparoa 0943

Phone number:

N/A N/A

Mobile number: Facsimile number:

N/A N/A

Email address:

mike@hookedonprint.co.nz N/A

Website:

Street address/registered office:

1830 Ngunguru Road Whangarei 0173

First point of contact for communications with council/building consent authority

Contact Person

Cook Costello Limited 2 Norfolk Street Whangarei 0110

Phone number:

4389529

Mobile number:

N/A

Facsimile number:

N/A mark@coco.co.nz

Email address:

Website:

N/A

Building Work

The following building work is authorised by this consent:

2 New Retaining Walls

This building consent is issued under section 51 of the Building Act 2004. This building consent does not relieve the owner of the building (or proposed building) of any duty or responsibility under any other Act relating to or affecting the building (or proposed building).

This building consent also does not permit the construction, alteration, demolition, or removal of the building (or proposed building) if that construction, alteration, demolition, or removal would be in breach of any other Act.

This building consent is subject to the following conditions:

Section 90 Building Act 2004

Under section 90 of the Building Act 2004, agents authorised by the Council (acting as a Building Consent Authority) are entitled, at all times during normal working hours or while building work is being

ii) land on which building work is being or is proposed to be carried out; and

iii) building work that has been or is being carried out on or off that building site; and

iiii) any building

See attached list of required inspections.

Compliance Schedule

A compliance schedule is not required for the building.

Attachments

The Project Information Memorandum for the building work covered by this building consent.

Additional Information

- Dust Nuisance The applicant must control dust nuisance created by any site or building works.
- Toilet Facilities Toilet facilities must be provided within reasonable distance of the construction site. Ground discharge is no longer acceptable.
- Location of council sewer line under northern wall to be accurately plotted and marked

Kayla Maxwell

Support Assistant - Building Consent Issuing

On behalf of: Whangarei District Council

19 October 2011

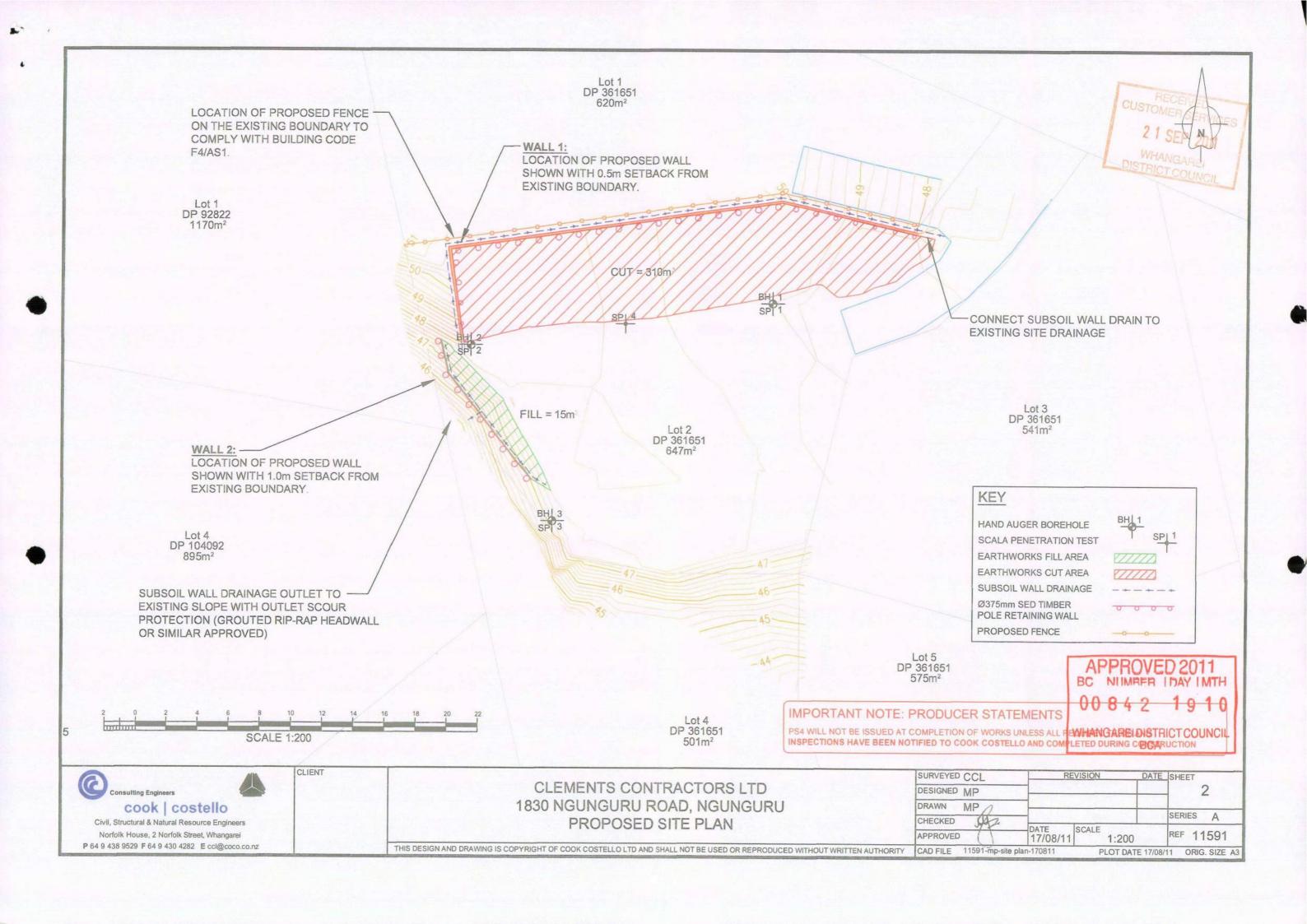


Table 1 Retaining wall dimensions

Wall	Height (max)	Pole Ø (mm)	Bore Ø 'b' (mm)	Spacing (m)	Embedment Depth 'd' (m)	Depth to Doubled Rails
1	3.0m	375	525	1.3m	4.0m min	1.0m
1	2.5m	325	475	1.3m	3.2m min	1.0m
1	2.0m	275	425	1.3m	2.5m min	1.0m
1	3.0m	225	375	1.3m	1.8m min	1.0m



-200mm topsoil cap to drainage material.

-150 x 75mm H5 RS timber rails. Rails fastened to the back of each pole with 2/100 x 4.0mm HDG FH nails. stagger timber rail joints at each row over poles.

7 - 20mm clean scoria or similar free draining coarse granular material to back of retaining wall. Drainage material to be wrapped with Terratex 160N (or equivalent approved) non-woven geotextile separation layer. Geotextile joints to be formed in accordance with Manufacturers requirements allowing for pipework penetrations.

Two sets of staggered rails at base of wall (refer to Table 1 for height of doubled-up rails)

110mm perforated drainage pipe to back of wall at bottom rail level. Minimum 100mm thick layer of scoria bedding material (or similar approved) to drainage pipe. Drain outlet to be connected to proposed piped site drainage.

High density H5 treated timber pole (spacing as per Table 1) raked at 6° (1h:10v). Paint all cut edges and ends of all treated timber with a copper napthenate based brush-on preservative.

Minimum 17.5Mpa concrete encasement with minimum 75mm cover to sides and base. Hole must be thoroughly cleaned out before placing concrete.

NOTE:

- 1. This drawing is not to be scaled
- 2. All timber poles to be high density
- 3. Timber pole cantilever retaining structure has been designed in accordance with:
 - -AS/NZS 1170
 - -NZS 3603
 - -NZBC B1/VM4
- 4. Design has been based on the following assumptions:
 - -Retained slope = 0° max
 - -Base slope = 0° max
 - -Retained surcharge = 20kpa
 - -Undrained shear strength cu=75.
 - -Retained soil effective friction angle = 28°

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WHANGAREIDISTRICT COUNCIL BCA

IMPORTANT NOTE: PRODUCER STATEMENTS

PS4 WILL NOT BE ISSUED AT COMPLETION OF WORKS UNLESS ALL REQUIRED TESTS AND INSPECTIONS HAVE BEEN NOTIFIED TO COOK COSTELLO AND COMPLETED DURING CONSTRUCTION

Consulting Engineers

cook | costello

Civil, Structural & Natural Resource Engineers
Norfolk House, 2 Norfolk Street, Whangarei
P 64 9 438 9529 F 64 9 430 4282 E ccl@coco.co.nz

CLIENT

CLEMENTS CONTRACTORS LTD 1830 NGUNGURU ROAD, NGUNGURU TYPICAL SECTION FOR RETAINING WALL 1

'b' (See Table1)

Typical Section for wall 1

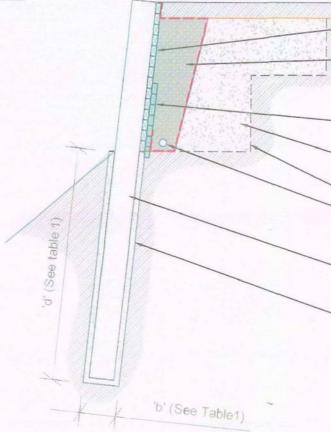
THIS DESIGN AND DRAWING IS COPYRIGHT OF COOK COSTELLO LTD AND SHALL NOT BE USED OR REPRODUCED WITHOUT WRITTEN AUTHORITY | CAD FILE | 11591-mp-site plan-170811

SURVEYED CCL REVISION DATE SHEET 3 DESIGNED MP DRAWN MP SERIES A CHECKED DATE SCALE REF 11591 APPROVED 17/08/11 NTS

AD FILE 11591-mp-site plan-170811 PLOT DATE 17/08/11 ORIG. SIZE A3

Table 1 Retaining wall dimensions

Wall	-	Bore Ø	Embedment	Depth to Doubled Rails
	375		3.6m min	



Typical Section for wall 2

-200mm topsoil cap to drainage material.

-150 x 50mm H5 RS timber rails. Rails fastened to the back of each pole with 2/100 x 4.0mm HDG FH nails. stagger timber rail joints at each row over poles.

-7 - 20mm clean scoria or similar free draining coarse granular material to back of retaining wall. Drainage material to be wrapped with Terratex 160N (or equivalent approved) non-woven geotextile separation layer. Geotextile joints to be formed in accordance with Manufacturers requirements allowing for pipework penetrations.

-Two sets of staggered rails at base of wall (refer to Table 1 for height of doubled-up rails)

Quarry run or similar well graded granular material as backfill to drainage layer, compacted in layers maximum 300mm thick.

Site benching as required for compaction of backfill.

110mm perforated drainage pipe to back of wall at bottom rail level. Minimum 100mm thick layer of scoria bedding material (or similar approved) to drainage pipe. Drain outlet to existing slope with outlet scour protection (grouted rip-rap headwall or similar approved).

-High density H5 treated timber pole (spacing as per Table 1) raked at 6° (1h:10v). Paint all cut edges and ends of all treated timber with a copper napthenate based brush-on preservative.

Minimum 17.5Mpa concrete encasement with minimum 75mm cover to sides and base. Hole must be thoroughly cleaned out before placing concrete.

NOTE:

- 1. This drawing is not to be scaled
- 2. All timber poles to be high density
- 3. Timber pole cantilever retaining structure has been designed in accordance with:
 - -AS/NZS 1170
 - -NZS 3603
 - -NZBC B1/VM4
- 4. Design has been based on the following assumptions:
 - -Retained slope = 0° max.
 - -Base slope = 40° max.
 - -Retained surcharge = 5kpa
 - -Undrained shear strength cu=75
 - -Retained soil effective friction angle = 28°

APPROVED 2011 BC NUMBER IDAY IMTH 00842 1910

CUSTOMER SERVICES

WHANGAREI DISTRICT COUNCIL BCA



PS4 WILL NOT BE ISSUED AT COMPLETION OF WORKS UNLESS ALL REQUIRED TESTS AND INSPECTIONS HAVE BEEN NOTIFIED TO COOK COSTELLO AND COMPLETED DURING CONSTRUCTION

Consulting Engineers

cook | costello

Civil, Structural & Natural Resource Engineers
Norfolk House, 2 Norfolk Street, Whangarei
P 64 9 438 9529 F 64 9 430 4282 E ccl@coco.co.nz

CLIENT

CLEMENTS CONTRACTORS LTD 1830 NGUNGURU ROAD, NGUNGURU TYPICAL SECTION FOR RETAINING WALL 2

THIS DESIGN AND DRAWING IS COPYRIGHT OF COOK COSTELLO LTD AND SHALL NOT BE USED OR REPRODUCED WITHOUT WRITTEN AUTHORITY

SURVEYED CCL

DESIGNED MP

DRAWN MP

CHECKED

APPROVED

DATE

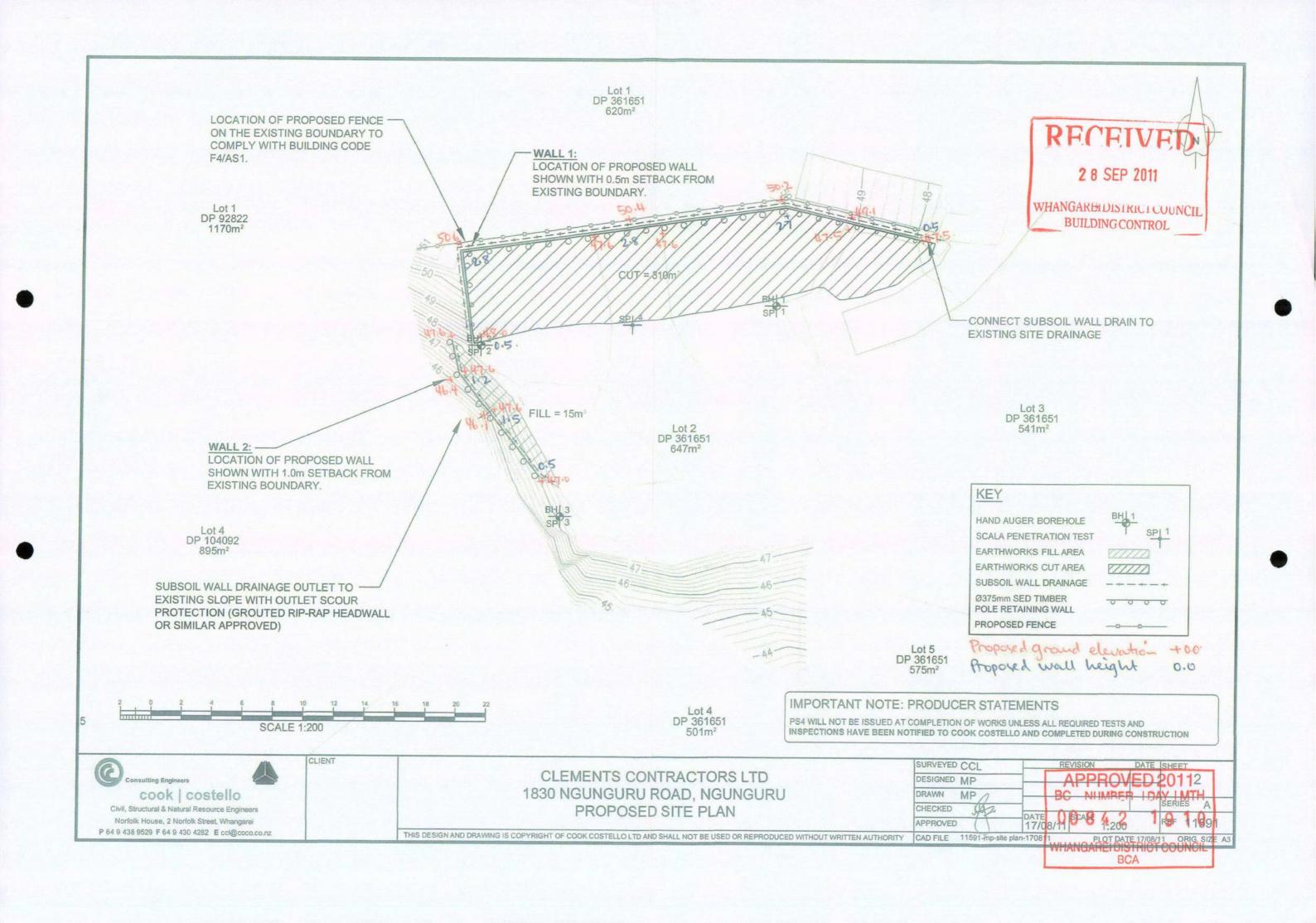
17/08/11

CAD FILE

11591-mp-site plan-170811

PLOT DATE 17/08/11

ORIG, SIZE A3







cook costello

Civil Structural Geotechnical Environmental Industrial Project Management

Norfolk House 2 Norfolk St Whangarei P 64 9 4389 529 F 64 9 4304 282 E ccl@coco.co.nz Kerikeri P 64 9 407 4409 F 64 9 407 4409 E ccl@coco.co.nz



Our ref: 11591 Your ref:

6 March 2011

Whangarei District Council Forum North, Private Bag 9023 Whangarei

Re: Design Summary - Retaining Walls, 1830 Ngunguru Road, Ngunguru

Dear Sir/Madam,

Regarding your client's (Mike Day) property at 1830 Ngunguru Road, Ngunguru; we write to confirm our design proposal for the proposed retaining walls.

1. Background Information

We have referred to the following information during preparation of the design:

- NZMS 290 Sheet Q 06/07, SOILS and ROCK TYPES.
- Site soil investigations by Cook Costello Ltd, August 2011 (see attached bore logs),
- Site topographical survey by Cook Costello Ltd, August 2011 (see attached site plan).

2. Ground investigations

Geotechnical field investigations were carried out at the site on 17 August 2011. The investigations consisted of:

APPROVED 2011

BC. NUMBER IDAY IMTH

00842 1910

WHANGAREI DISTRICT COUNCIL BCA

- · Visual inspection, walkover and site photographs.
- Four Dynamic Cone Penetrometer (scala) tests,
- Three hand auger boreholes (BH) between 2m and 3m deep, to identify subsurface soil properties and the presence of groundwater.

The locations of field tests are shown on drawing 11591-A-1 (attached). Scala and Borehole reductions are also attached.

Soil recovered from BH1 (1.6m deep) was generally described as very silty CLAY; orange or brown, mottled white; slight moisture increasing with depth, sensitivity decreasing from quick to sensitive at depth. Groundwater was not found below existing ground level.

Soil recovered from BH2 (2.5m deep) was generally described as silty CLAY; orange brown, mottled orange and white to uniform colour at depth; wet, sensitivity decreasing from quick to moderately sensitive at depth. Groundwater was not found in BH2.

Soil recovered from BH3 (3.0m deep) was generally described as silty CLAY; orange brown, mottled white to uniform colour at depth; moist, sensitivity decreasing from extra sensitive to moderately sensitive at depth. Groundwater was not found in BH3.

2. Assumptions

In order to produce this design, we have made some assumptions based on our current understanding of the project objectives. These assumptions are as follows:

Wall 1;

- 1. The maximum retained height of the wall will be 3.0m,
- 2. Wall foundation soil (cohesive) parameters: γ=18kN/m³; Cu=75kPa,
- 3. Retained fill material (cohesive) soil parameters: γ=17kN/m³; φ'=28°,
- 4. Backfill behind the wall will be compacted granular fill, including clean granular drainage material with perforated drain coil immediately behind the wall,
- 5. An allowance in the design of 20kPa surcharge for future building on the neighbouring property (1828 Ngunguru Road),
- 6. The visual impact of bulge in the wall railings is required to be minimal.

Wall 2:

- 7. The maximum retained height of the wall will be 1.5m,
- 8. Wall foundation soil (cohesive) parameters: γ=18kN/m³; Cu=75kPa,
- Retained fill material (cohesive) soil parameters: γ=17kN/m³; φ'=28°,
- Backfill behind the wall will be compacted granular fill, including clean granular drainage material with perforated drain coil immediately behind the wall.
- 11. An allowance in the design of 5kPa surcharge for light loading above the wall,
- 12. The visual impact of bulge in the wall railings is required to be minimal.

However, if it becomes apparent that any of the assumption above are incorrect (either prior to or during construction), please inform the engineer immediately as this may affect the wall design.

3. Recommended Design

Retaining wall 1 to the north of the property:



- Maximum wall height 3.0m, 7.0m long 375mm S.E.D high density timber poles to H5 treatment, at 1.3m spacing.
- Minimum pole embedment depth 4.0m.
- Railings to be 150×75mm; single railing to 1.0m depth of fill, double railing for fill depth greater than 1.0m.

Retaining wall 2 to the west of the property:

- Maximum wall height 1.5m, 5.1m long 375mm S.E.D high density timber poles to H5 treatment, at 1.2m spacing.
- Minimum pole embedment depth 3.6m.
- Railings to be 150×50mm; single railing to 0.5m depth of fill, double railing for fill depth greater than 0.5m.

The attached drawing (11591-A-3 & 11591-A-4) provides construction details for the proposed retaining wall maximum heights for retaining wall 1 of 3.0m, 2.5m, 2.0m and 1.5m should your client wish to use poles of varying diameters.

Please note that doubling up of railings can be reduced if the visual impact of rail bulge is acceptable to the client.

In addition to the design, in if a PS4 is required from us for the construction work, we will need to inspect the works at the following stages:

- 1. Excavation of foundation holes,
- Installation of poles prior to placement of concrete,
- Construction of railings & installation of drainage to the back of the wall and drainage material,
- 4. Compaction of granular backfill to wall 2.

Please contact me if you have any questions regarding the above.

Yours faithfully,

Mark Shaw

Chartered Professional Engineer CPEng, MIPENZ, CEng(UK), MInstP





Forum North, Private Bag 9023 Whangarei 0148, New Zealand Telephone: +64 9 430 4200 Facsimile: +64 9 438 7632 Email: mailroom@wdc.govt.nz Website: www.wdc.govt.nz

Code Compliance Certificate BC1100842 Section 95, Building Act 2004 Issued: 02 April 2012

The Building

Street Address of building: 1830 Ngunguru Road

Whangarei 0173

Legal Description of land where building is located: LOT 2 DP 361651

LLP: 114627

Building name:

Location of building within site/block number:

N/A

Level unit number:

N/A

Current, lawfully established use:

Year first constructed:

N/A

2011

The Owner

M J Day M Day 1830 Ngunguru Road Whangarei 0173

Phone number: N/A
Mobile number: N/A
Facsimile number: N/A
Email address: N/A
Website: N/A

First point of contact for communications with the building consent authority:

Contact Person

M J Day 1830 Ngunguru Road Whangarei 0173

Phone number: N/A
Mobile number: N/A
Facsimile number: N/A
Email address: N/A
Website: N/A

Street address/registered office: 1830 Ngunguru Road

Whangarei 0173

Building Work

Building Consent number

Issued by:

2 New Retaining Walls

BC1100842

Whangarei District Council

Code Compliance

The building consent authority named below is satisfied, on reasonable grounds, that -

(a) The building work complies with the building consent.

Kylee Akast

Support Assistant - Code Compliance

On behalf of: Whangarei District Council

02 April 2012



Building Consent No: BC1700484

Section 51, Building Act 2004

Issued: 10 July 2017

Project Information Memorandum No: PM1700116

The Building

Street address of building: 1830 Ngunguru Road

Whangarei 0173

Legal description of land where building is located: LOT 2 DP 361651

LLP: 116427

N/A

Building name: N/A

Location of building within site/block number: N/A

Level/unit number:

The Owner

K A Stubbs P S Stubbs

361 Matapouri Road

RD 3

Whangarei 0173

Phone number: N/A
Mobile number: N/A
Facsimile number: N/A

Email address: N/A

Website: N/A

Street address/registered office: 1830 Ngunguru Road

Whangarei 0173

First point of contact for communications with Council/building consent authority

Contact Person

A1 Homes Northland

PO Box 183 Ruakaka 0151

Phone number: 4330200

Mobile number: 021588351 021729724

Facsimile number: 4330209

Email address: mark.russell@a1homes.co.nz

Website: N/A

Building Work

The following building work is authorised by this consent:

New Dwelling



This building consent is issued under section 51 of the Building Act 2004. This building consent does not relieve the owner of the building (or proposed building) of any duty or responsibility under any other Act relating to or affecting the building (or proposed building).

This building consent also does not permit the construction, alteration, demolition, or removal of the building (or proposed building) if that construction, alteration, demolition, or removal would be in breach of any other Act.

This building consent is subject to the following conditions:

Section 90 Building Act 2004

Under section 90 of the Building Act 2004, agents authorised by Council (acting as a Building Consent Authority) are entitled, at all times during normal working hours or while building work is being done, to inspect:

- ii) land on which building work is being or is proposed to be carried out; and
- iii) building work that has been or is being carried out on or off that building site; and
- iiii) any building.
- 1. See attached schedule of site requirements for inspections and documentation required.
- A copy of your Electrical Certificate will be required.
- 3. A Producer Statement PS4 is required for palisade walls.
- 4. A Producer Statement PS4 is required for compacted fill.
- 5. A Producer Statement PS4 from CPENG is required for the inspection of pile excavation to ensure foundation extend through the fill and pin into natural ground beneath; and confirm ground condition as anticipated and provide advice as appropriate for acid sulphate risks.

Compliance Schedule

A compliance schedule is not required for the building.

Attachments

No attachments.

Additional Information

The applicant must control dust nuisance created by any site or building works.

Toilet facilities must be provided within reasonable distance of the construction site. Ground discharge is no longer acceptable.

 Lapsing of building consent. For the purposes of S52(b) of the Building Act 2004, the period after which this consent will lapse if the building work to which it relates does not commence will be 12 months from the date of issue.

Builder to ensure that palisade wall clearance from council SS manhole is a per approved plan.

Eboylan.	10 July 2017
Enka Boylan	Date
Support Assistant – Building Processing	
On behalf of Whangarei District Council	



NZBC F5: Construction and Demolition Hazards Acceptable Solution F5/AS1

1.0 Work-Site Barriers

1.0.1 The necessity for barriers will depend mainly on the site location.

The need will be greater in areas with high levels of pedestrian traffic (i.e. in Central Business Districts), than in industrial or rural areas.

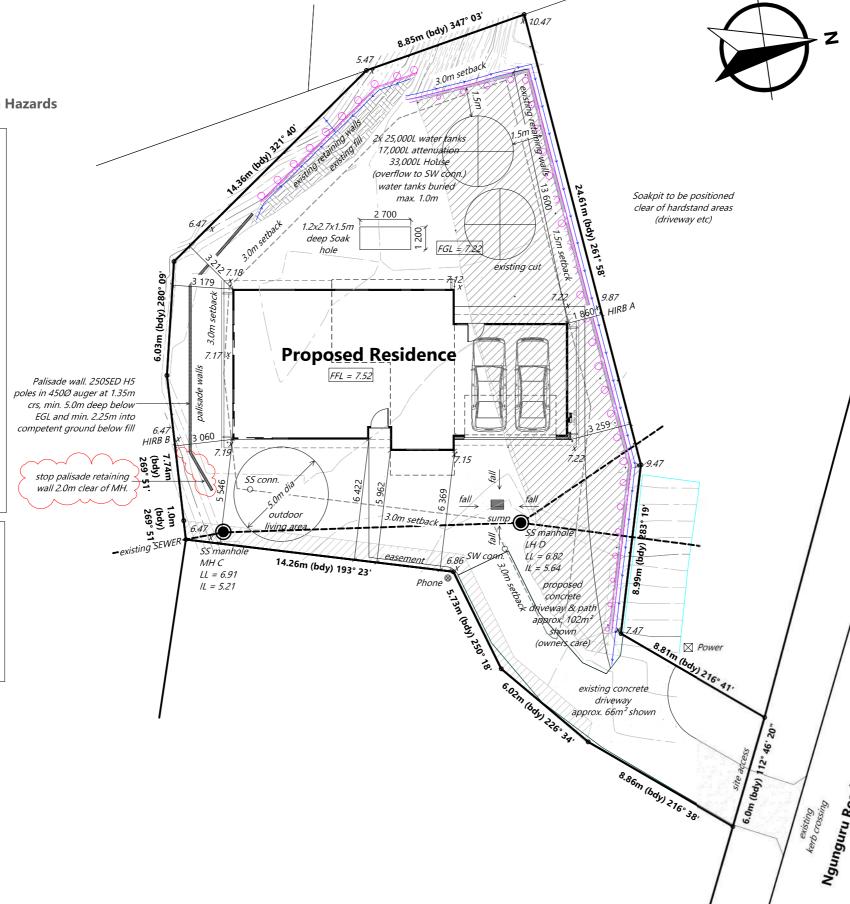
Barriers are not necessary for domestic dwellings up to 2 storeys above ground level unless specific hazards exist.

At all work-sites hazard evaluation will take account of

- 1. Pedestrian counts adjacent to the site.
- 2. Car parking adjacent to the site.
- 3. Location of neighbouring buildings.
- 4. Presence of neighbouring work-sites or recreation areas.
- 5. Proximity to schools or early childhood centres.
- 6. Proximity to housing.
- 7. The depth of a water hazard.
- 8. The period of time for which ponded water will be present.
- 9. The accessibility and 'visibility' of the site.
- 1.0.2 If a work-site is not completely enclosed, and unauthorised entry by children is likely, it is acceptable for specific hazards to be fenced only when workers are absent from the immediate vicinity.

1.1 Site fences and hoardings

- 1.1.1 Fences and hoardings shall extend at least 2.0 m in height from ground level on the side accessible to the public.
- 1.1.2 An acceptable fence may be constructed with galvanised chainlink netting having a maximum sized grid of 50 mm x 50 mm. Post spacing shall be a maximum of 2.5 m, and the gap between the bottom of the fence and ground no greater than 100 mm.





Cautionary Notes:

BUILDING CONTRACTOR TO ASSESS SITE TO ENSURE DAYLIGHTING & BUILDING RESTRICTIONS ARE COMPLIED WITH.

NO LIABILITY FOR ENCROACHMENT SHALL BE HELD BY DESIGNER IF SITE IS NOT SURVEYED BY A REGISTERED SURVEYOR PRIOR TO COMMENCEMENT OF FOUNDATIONS.

Construction Notes:

Before building is erected on site, all rubbish, noxious matter and organic matter shall be removed from the area to be covered by the building.

Ensure final building platform & finished ground have an even fall away from building to ensure water not be allowed to accumulate in buildings subfloor.

Any fill to be dry & approved by engineer & compacted down in accordance with NZS.3604.2011

Contractor to

- confirm ground has adequate bearing to comply with NZS 3604: 2011, except in the case of SED design, or when using Firth RibRaft Floor System (refer Ribraft manual).
- locate all service connections points on site prior to commencement of works. Check invert levels or pipes and manholes.
- confirm plumbing route and fixture positions on site prior to commencement of works.
- locate all electrical and water services on site.
- confirm on site all boundary bearings, lengths & peg locations on site prior to commencement of works, to ensure house position is correct.

HIRB = Height in Relation to Boundary

Sediment Control:

• No building work will be started on this project until the construction of an approved stormwater outfall has been completed for this proposed Lot

- All erosion and sediment control structures are to be inspected and maintained daily
- Prevent any backfill or debris from washing onto council or neighbouring properties
- All ground cover vegetation outside the immediate building area to be preserved during the building phase
- All erosion and sediment control measures are to be installed prior to commencement of earthworks
- · Stockpiles of clay and materials are to be covered with impervious sheeting
- Roof water downpipes to be connected to the main stormwater system as soon as roof sheathing & spouting is installed

Lot: 2 DP: 361651 Site Area: 647m²

Gross Rian Area: 140.0m²

(incl. cover Entry & 1F o/hang)

PO549

Site Coverage: 21.6% (max 35%)

Maximum Building Ht: 8m

Territorial Authority: Whangarei District Council

Planning Zone: Living 1
Client Details:

Phillip Stubbs Address: 1830 Ngunguru Road Ngunguru

	Sit	e Plan]	Scale: 1:200	Date: 6/07/2017 Rev: D: 06-July-17	7 -
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DO NOT scale off drawings. Cross reference all drawings. Any discrepancies MUST be clarified with the designer immediately before commencing works or ordering. NO construction or site works are to commence until Building Consent becomes unconditional. COPYRIGHT: Any and all drawings commissioned remain the property of A1 Homes Limited.

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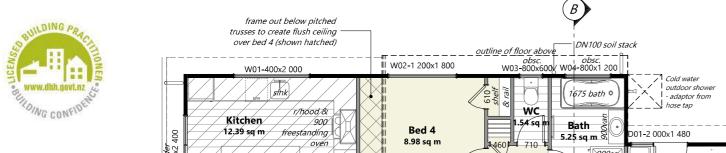
Finished Floor Level (FFL) shall be:

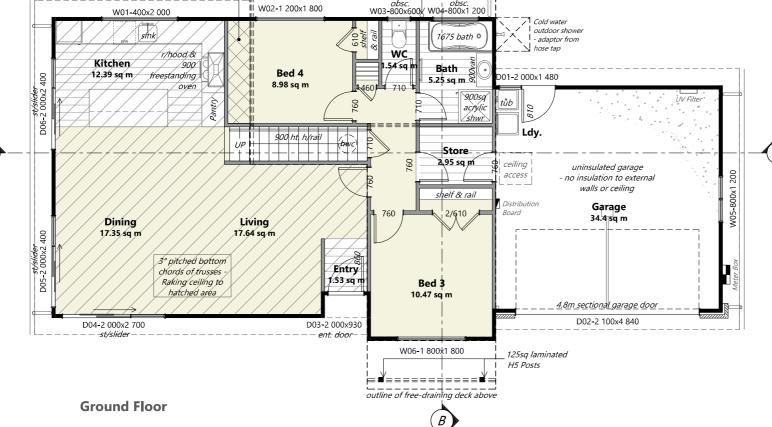
- a) For sites level with or above the road, no less than 150 mm above the road crown on at least one cross-section through the building and roadway
- b) For sites below the road, no less than 150 mm above the lowest point on the site boundary

(a) protected from contamination; and
 (b) installed in a manner which avoids the likelihood of contamination within the system and the water main; and
 (c) installed using components that will not contaminate the water.

A potable water supply system shall be-

FFL to be min 2.5m above One Tree Point Datum Mean Sea Level 1964

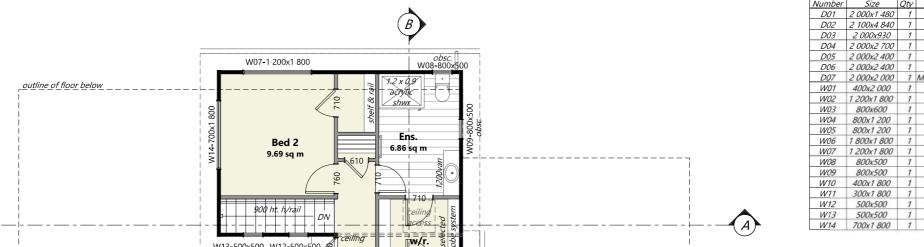




W13-500x500 W12-500x500

(2190 head ht.)

access



3.77 sq m

Master Bed

11.91 sq m

Deck

4.39 sq m

(B)

Cladding Key:

JH 180 Linea w/board

Exterior Joinany Cohodule

Firth 100S Architectural Honed Block - Stack Bond

Number	Size	Qty	Room Name
D01	2 000x1 480	1	Garage
D02	2 100x4 840	1	Garage
D03	2 000x930	1	Entry
D04	2 000x2 700	1	Dining
D05	2 000x2 400	1	Dining
D06	2 000x2 400	1	Kitchen
D07	2 000x2 000	1	Master Bed/Deck
W01	400x2 000	1	Kitchen
W02	1 200x1 800	1	Bed 4
W03	800x600	1	WC
W04	800x1 200	1	Bath
W05	800x1 200	1	Garage
W06	1 800x1 800	1	Bed 3
W07	1 200x1 800	1	Bed 2
W08	800x500	1	Ens.
W09	800x500	1	Ens.
W10	400x1 800	1	Master Bed
W11	300x1 800	1	Master Bed
W12	500x500	1	Stairwell
W13	500x500	1	Stairwell
W14	700x1 800	1	Bed 2

Floor Finishes:

Carpet = $96.9m^2$ Wet Area = 30.8m² Bathroom Bed 3 Bed 4 WC Livina Kitchen Dining Entry Store

Hallways, cupd.s & wdrbs Master Bed Bed 2

Concrete = 34.4m²

Garage Laundry

Ensuite

outline of floor below

1.0m ht. eng. semi

(by others)

etch glass balustrade

1.1.3 Food preparation surfaces shall be easily maintained in a hygienic condition. Stainless steel, decorative high pressure laminate, and tiles are examples of suitable materials for these surfaces.

1.6 Wall linings

Wall linings adjacent to appliances and facilities shall have surfaces that can be easily maintained in a hygienic condition. Stainless steel, decorative high pressure laminate, tiles, wallboards with painted or applied impervious coatings or films, are examples of suitable materials for these surfaces.



Cautionary Notes:

Always cross reference the foundation plan with the framing plan prior to setting out

Joinery sizes shown are box sizes & are preliminary only.

Site measure and confirm all joinery sizes, reporting to designer any changes, PRIOR to ordering joinery. No liability shall be held by designer for incorrect supply of

Refer to all written dimensions, DO NOT scale off drawings.

Construction Notes:

Electric hobs with vented r/hood.

Polybutylene water supply pipes.

Hot water supply pipes shall be thermally insulated to comply with H1/AS1 5.0 Mains pressure 180L HWC with tempering valve & seismic restraint in accordance with NZBC: 2004 section G12.

The delivered hot water temperature at any sanitary fixture used for personal hygiene shall not exceed 55°C

Tapered edge joints in ceilings

To reduce the risk of cracks caused by substrate movement, back-blocking of tapered edge joints is required in the following situations.

- When timber battens have been used:
- Any area containing 3 or more tapered joints
- When steel battens have been used:

Any area containing 6 or more tapered joints

Please confirm layout & fittings of kitchen & bathrooms etc before foundation

A landing min 900deep shall be provided at the top & bottom of every flight of stairs where the rise of the flight is more than 600mm. Handrails are required to one side of all stairs with 4 or more risers - NZBC D1/AS1: Access routes

Separation between electric hob and the Gib lined wall: Cut out for hob: min. 55mm from back of bench top. Overhead clearances: not less than 650mm from hob surface to range

Side clearances: Where dimension to any vertical combustible surface is less than 150 mm, surface shall be protected to a min. height of 150 mm above hob for full dimension (width or depth) of cooking surface area. Protection of combustible surfaces: 5mm thick ceramic tiles or graphic glass is suitable to protect 10mm Gib board.

Stairs to comply with NZBC:D1 access; main private max 190rise, min 280 tread. Wall mounted grab rail @900ht from tread nosing

Ground Floor Area:

- = 130.8sqm o/frame (incl. stairwell)
- = 131.7sqm o/brick (incl. stairwell)

First Floor Area:

Client Details :

Phillip Stubbs

Address:

3.3sqm o/frame (excl. stairwell = 3.3sqm)

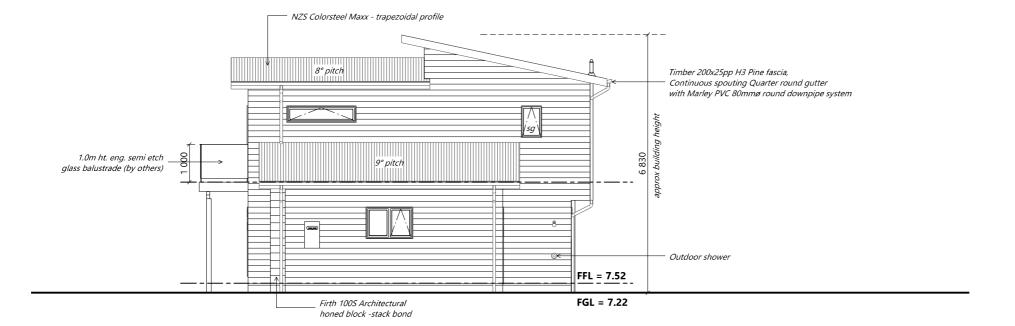
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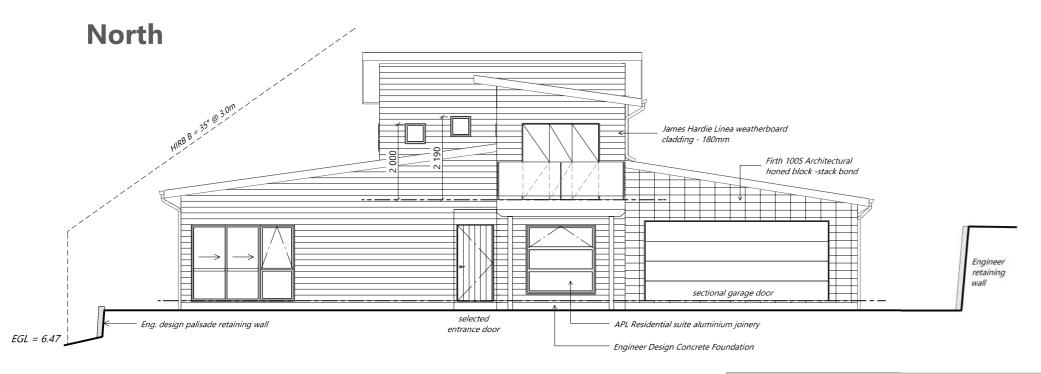
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First Floor







East

NZBC D1/AS1 Access Routes:

Concrete (min 150mm below FFL) or H5 timber step to all access points (owners care)

Acceptable Slip Resistance for Walking Surfaces:

- Portland cement concrete
- Broomed (Class 5 or 6) or wood float finish (Class U2)

Concrete surface finishes complying with NZS 3114.

- Coated and sand/grit impregnated

The sand/grit, which is sprinkled over the complete surface of the final paint coating, should be a hard angular material such as silica sand or calcined bauxite. The particle size should not be less than 0.2 mm so that it is not submerged by the coating and not greater than about 2–3 mm so that it remains tightly bound to the surface.

- Exposed aggregate finish

crushed aggregate

- Asphaltic concrete
- Concrete pavers
- Dry press concrete
- Interlocking concrete block paving to NZS 3116.
- Anti-slip tapes

 will normally require regular replacement to remain effective. To ensure foot contact, tapes should be placed at right angles to the line of travel and be spaced at no more than 150 mm centres.



Cautionary Notes:

BUILDING CONTRACTOR TO ASSESS SITE TO ENSURE DAYLIGHTING & BUILDING RESTRICTIONS ARE COMPLIED WITH.

NO LIABILITY FOR ENCROACHMENT SHALL BE HELD BY DESIGNER IF SITE IS NOT SURVEYED BY A REGISTERED SURVEYOR PRIOR TO COMMENCEMENT OF FOUNDATIONS.

Construction Notes:

Glazing in accordance with NZS 4223 & 2008 plus amendments All glazing low-e clear float except for obscure glass to bathrooms & wc Double glazing to all window and door joinery excluding garage sg = Safety glass

Aluminium joinery head heights to be 2.0m unless noted otherwise (excludes entry box unit).

Refer to floor plan for door & window sizes. Joinery schedule & sizes to be confirmed by pre-cut manufacturer & joinery fabricator PRIOR to manufacture by way of communication via e-mail, phone or other.

HIRB = Height in Relation to Boundary

Safety restrictor stays:

ss = safety stays

- a restrictor fitted to limit the maximum opening so that a $100 \text{mm}\text{\ensuremath{\emptyset}}$ sphere cannot pass through

Window restrictors are required to the following sized openings where the adjacent Ground Level is 1.0m below FFL or greater;

- openings less than 1.0m wide with sill ht within 760mm of FFL
- openings more than 1.0m wide with sill ht within 1000mm of FFL

Window restrictors are also required to outward opening windows that may protrude into walk paths

- Refer to Site plan for 'walk paths'

Building Envelope Risk Matrix

All Elevations

Risk Factor
Wind zone (per NZS 3604)
Number of storeys
Roof/wall intersection design
Eaves width
Envelope complexity
Deck design

All Elevations

Risk Severity
N/High risk
Medium risk
Medium risk
1
Medium risk
1
Medium risk
1
Medium risk
1
Medium risk
2
Total Risk Score: 12

Client Details:
Phillip Stubbs
Address:
1830 Ngunguru Road
Ngunguru

Scale:

Elevations

Date: 6/07/2017
Sheet no:
1:100
Rev:
9

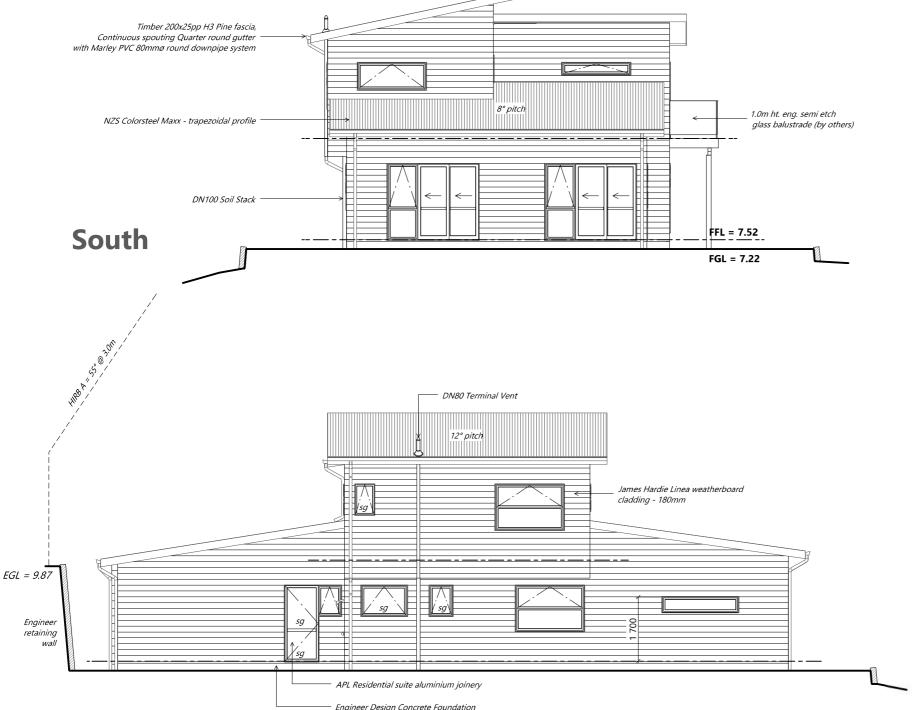
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- will normally require regular replacement to remain effective. To ensure foot contact, tapes should be placed at right angles to the line of travel and be spaced at no more than 150 mm centres.



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HIRB = Height in Relation to Boundary

Safety restrictor stays:

ss = safety stays

Client Details :

- a restrictor fitted to limit the maximum opening so that a 100mmø sphere cannot pass through

Window restrictors are required to the following sized openings where the adjacent Ground Level is 1.0m below FFL or greater;

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Building Envelope Risk Matrix

All Elevations

Risk Factor
Wind zone (per NZS 3604)
Number of storeys
Roof/wall intersection design
Eaves width
Envelope complexity
Deck design

All Elevations
Risk Severity
V/High risk
Medium risk
Medium risk
1
Medium risk
1
Medium risk
2
Total Risk Score: 12

Phillip Stubbs Address: 1830 Ngunguru Road Ngunguru			BH172
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West





Geotechnical Report

1830 Ngunguru Road, Ngunguru Lot 2 DP 361651

For Phillip Stubbs

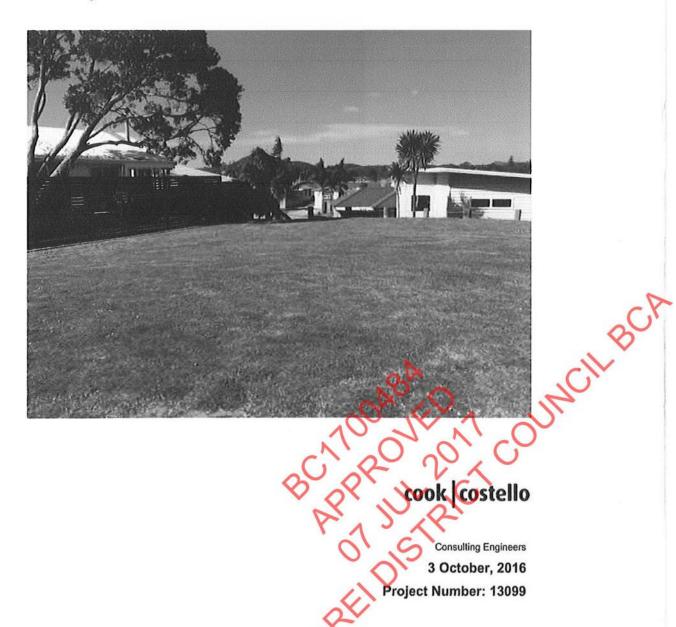


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WHANCARE INSTRUCT

1. INTRODUCTION

It is proposed to construct a new residential dwelling at the property of Phillip Stubbs on Lot 2 DP 361651, 1830 Ngunguru Road, Ngunguru.

Cook Costello have been briefed to provide a geotechnical report for the proposed development. This report considers the following aspects of site development:

- Existing stability of the site
- Effects of the development on stability
- Stormwater management
- Suitable building foundations
- Assessment of the stability of the building site is terms of Section 72 of the Building Act, 2004

A site plan is attached in Appendix 1 showing the property boundaries and associated site investigations.

1.1. Relevant Documentation

- AS 2870: 2011 Construction of residential slabs and footings
- NZS 3604: 2011 Timber framed buildings
- NZS 4402:1986 Methods of testing soils for civil engineering purposes.
- NZ Building Code: B1/VM4
 - Good Ground means any soil or rock capable of permanently withstanding an ultimate bearing pressure of 300kPa (i.e. an allowable bearing of 100kPa using a factor of safety of 3.0) but excludes;
- a) Potentially compressible ground such as topsoil, soft soils such as clay which can be moulded easily in the fingers, and uncompacted loose gravel which contains obvious voids,
- b) Expansive soils being those that have a liquid limit of more than 50% when tested in accordance with NZS4402 Test 2.2 and a linear shrinkage of more than 15% when tested from the liquid limit in accordance with NZS 4402 Test 2.6 and,
- c) Any ground which could forseeably experience movement of 25mm or greater for any reason including one or a combination of the following: land instability, ground creep, subsidence, seasonal swelling and shrinking, frost heave,

INTRODUCTION

1

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changing ground water level, erosion, dissolution of soil in water, and effects of tree roots.

Whangarei District Council: 2016 – GIS Maps

2. SITE DESCRIPTION

The property is located at 1830 Ngunguru Road in Ngunguru. The site is accessed via a shared driveway off Ngunguru Road to the north of the site. The subject site is set one property back from Ngunguru Road. The legal description of the site is Lot 2 DP 361651 and the total size of the lot is 647m².



Figure 1. Aerial photograph showing the subject property

The property has a flat contour, however earthworks have been undertaken on the site. It appears that historically there has been some cut undertaken on the northern side of the property with fill placed on the southern side of the site. In 2011 further earthworks involving cut along the northern property boundary were undertaken with the cut being retained. Fill was placed on the western side of the site, with a retaining wall constructed along most of the western property boundary. There is a steep embankment along the southern property boundary where the land slopes down to the south at angles up to 45°. There is a 2m high timber retaining wall at the base of the steep embankment, within the neighbouring property to the south.

The proposed construction of the new dwelling is a lightweight timber structure with linea weatherboard cladding. The proposed foundation system is unknown at the time of this report. The proposed dwelling may be single or double storey.

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SITE DESCRIPTION

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There was no evidence of any slips or land subsidence within the property or surrounding properties at the time of the site visit.

Whangarei District Council hazard maps show the property lies within high, medium and low risk stability areas. The high risk area is restricted to the western side of the property. The majority of the proposed building platform is within the medium risk instability zone, while the driveway is zoned as low risk of instability. The neighbouring property to the south is located within a flood zone. A minor portion of the subject property, along the southern lot boundary, is located within the flood zone.



Figure 2. WDC hazard map showing stability risk and flood susceptibility

Flood susceptible
High instability
Medium instability
Low instability

GEOLOGY

The soil type in the area is defined on NZMS290 Sheet Q06/07 Hukerenui Whangarei (SOILS) as Marua clay loam; well to moderately well drained.

The rock type in the area is defined on NZMS290 Q06.07 Hukerenui - Whangarei (ROCK TYPES) as A1₂: Alluvium: mud, sand and gravel with minor peat, forming river bed and flood plain deposits up to 60m thick; unconsolidated to very soft. Unweathered.

The Institute of Geological and Nuclear Sciences Geology of the Whangarei Area define the geology of the site as unconsolidated to poorly consolidated mud, sand, gravel and peat deposits of alluvial swamp and estuarine origin.

GEOLOGY

3

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rootlets from approximately 1.7mbgl, suggesting some degree of uncontrolled fill along the southern side of the lot.

Test ID	Start Depth (mBGL) ¹	Depth (mBGL) ¹	Scala Penetrometer (mm/Blow)	Inferred Ultimate Bearing Capacity (kPa)
SP1	1.60	1.60	<28mm/blow	>300
SP2	2.50	2.50	<28mm/blow	>300
		3.37	>50mm/blow	<200
SP3	3.00	3.87	<50mm/blow	>200
		4.01	<28mm/blow	>300
SP4	0.00	0.06	<28mm/blow	>300
SP5	0.00	0.07	<50mm/blow	>200
355 0.00	0.16	<28mm/blow	>300	
SP6	0.00	0.10	≤50mm/blow	≥200
576 0.00	0.14	<28mm/blow	>300	
		0.13	>50mm/blow	<200
		0.25	<28mm/blow	>300
SP7	0.00	0.48	<50mm/blow	>200
SF7	0.00	2.70	>50mm/blow	<200
		3.30	<50mm/blow	>200
		4.33	<28mm/blow	>300
		0.08	>50mm/blow	<200
SP8	0.00	1.89	<50mm/blow	>200
		2.15	<28mm/blow	>300

¹ mBGL: metre Below Ground Level

Table 2. Summary of Scala penetrometer results

Scala penetrometer results show that an ultimate bearing capacity (UBC) in excess of 300kPa (100kPa allowable) is available from approximately 0.2m below ground within the cut area of the site. However UBC >300kPa is identified from approximately 4.4m below the existing ground level within the area of expected uncontrolled fill on the southern side of the lot (worst case SP7).

Uncorrected bearing capacities derived from Scala penetrometer tests were estimated using the procedure presented by M.J. Stockwell in the paper 'Determination of allowable bearing pressure under small structures (June 1977)'.

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5. SITE STABILITY

The proposed building platform is flat with some cut to fill undertaken on site. It appears that some historical cut and filling has been undertaken on the site, with some uncontrolled fill placed on the southern side of the subject property. Further cut along the northern property boundary was undertaken in 2011 which has been retained. Some of the excess cut material was compacted along the western property boundary and this was also retained. The southern property boundary slopes down steeply to the neighbouring property below to the south at slope angles up to 45°. This appears to be the historical uncontrolled fill embankment.

No evidence of instability was observed within the property at the time of the site inspection. The site is considered to be consistent with the above geological description and is underlain by soils interpreted as residual weathered alluvial silts and sands. The site contains a building site that is considered suitable for the development as proposed. The proposed development is unlikely to have a detrimental effect on the site stability, provided the development is carried out in a responsible manner and in accordance with recommendations stated within this report.

6. STORMWATER

All stormwater resulting from development works and newly formed impermeable surfaces for the property, including overflow from roof water collection tanks shall be collected and piped to the Council stormwater system within Ngunguru Road.

In no instance is concentrated storm water to be discharged onto slopes without being specifically assessed, as this will be detrimental to slope stability.

6.1. Stormwater Attenuation

Whangarei District Council requires attenuation of stormwater runoff in certain circumstances, and for this proposed development it is anticipated that attenuation will be required. WDC requires that the runoff from the developed site is to be less than 80% of the existing site including an allowance of 20% for climate change for a 100 year rainfall event. The most convenient means to achieve this is to collect roof runoff from the buildings and direct to a storage tank for attenuation.

This method can also be used to compensate for other impervious surfaces which cannot be readily collected and attenuated (i.e. driveways, paved areas, paths).

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SITE STABILITY

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Specific design of stormwater attenuation may be required once the layout and final configuration of the development is established and can be completed as part of the Building Consent application.

6.2. Stormwater Tank Installation

Stormwater tanks are proposed to be buried at the site. These may be situated as close to Retaining Wall 2 (along the western property boundary) as practicable however must be offset from Retaining Wall 1 situated along the northern property boundary. To ensure the integrity of the retaining wall is maintained the tanks must be offset by a minimum of the height of the wall; this would be 3m from Retaining Wall 1. Alternatively, specific design of the tanks including bracing of Retaining Wall 1 may reduce the offset required.

7. FLOOD SUSCEPTIBILITY

The southern edge of the subject property is located within an area indicated as flood susceptible within Whangarei District Councils GIS maps. However the flood zone will be restricted to the base of the fill embankment; the proposed building site is elevated approximately 4-4.5m above this level. Therefore flooding is not considered to be an issue at the site.

8. SLOPE STABILITY ANALYSIS

8.1. Site Stability

The likelihood of slope failure is quantified by means of a Factor of Safety and is determined by the ratio of stabilising forces to destabilising forces. An acceptable slope will generally have a factor safety of 1.2 to 1.5 with a normal FOS value of 1.5 for subdivisions or housing development. The factors of safety adopted by engineers in geotechnical design have been developed to accommodate uncertainties in geometric accuracy, soil properties, analysis method, and the validity of assumptions made.

The modelled FOS does not assure safety from instability or slope movement but indicates a reduced likelihood of failure. The likelihood of failure for different levels of Factors of Safety is approximately:

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Factor of Safety	Likelihood of Failure Per Annum
1.1	1:10
1.3	1:50
1.5	1:200
1.7	1:1000

Table 3. Slope stability likelihood of failure

Generally the higher the risk category for the asset under consideration, the higher the design FOS to be adopted. The likelihood of slope failure was modelled as a circular failure using SLIDE analysis software to determine the existing stability of the slope under various conditions and to assess the likelihood of failure affecting the proposed dwelling.

The likelihood of any slope failure is dependent on the ratio of forces causing and resisting movement. Factors causing movement include the slope gradient, weight of soil, ground water, surcharge, and the factors resisting movement include slope support, soil strength parameters. Groundwater plays a critical role in slope stability, and soil shear strength when wet may be reduced to less than half of the strength when dry.

8.2. Soil Parameters

The soil parameters used for slope stability analysis are tabulated below.

Soil Type	Density (γ) kN/m³	Effective Cohesion c' kPa	Effective Friction Angle Φ' deg
Silty CLAY	18	5	25
Uncontrolled FILL	18	2	22
Weathered Alluvial Deposits	22	10	35

Table 4. Soil parameters used for slope stability analysis

These parameters have been selected based on the materials encountered on site, and engineering judgement.

8.3. Slope Stability Analysis

A typical slope stability analysis through Section A was undertaken using a combination of contour data obtained in 2011 at the time of construction of the retaining walls and clinometer data measured through the uncontrolled fill embankment along the southern boundary. Section A was analysed using SLIDE software to establish the risk of slope failure, set back distances from the edge of steep slopes and recommended foundation treatments.

The location of Section A used for the stability analysis is indicated on the site plan attached as Appendix 1. Slope stability results are attached as Appendix 3.

8.3.1. Existing Surface

The existing surface of the site has been modelled with the existing retaining wall at the back of the building platform, uncontrolled fill embankment at the front and neighbouring retaining wall below. Balanced cut to fill has been assumed on site, with uncontrolled fill from historical cuts on site placed at the front of the building platform.

The existing surface of the site was modelled under normal groundwater conditions. with the water table assumed just below the weathered alluvial deposit interface at approximately 5.5m below ground level at the front of the building platform.

The analysis shows failure surfaces with a Factor of Safety (FOS) <1.5 extending approximately 2.6m into the site from the top of the fill embankment. The lowest FOS occurring through the steep fill embankment is shown as 1.1.

The existing surface was also modelled under adverse groundwater conditions where the ground surface is saturated following a heavy rainfall event or failed drainage. Failures surfaces with FOS <1.2 extend approximately 6.7m into the site from the top of the fill embankment. The lowest FOS occurring through the steep fill embankment is shown as 0.6.

8.3.2. Finished Surface

The finished surface was modelled with a uniformly distributed load of 20kN/m² applied at the proposed building platform to represent the future dwelling.

Under normal groundwater conditions and assuming a concrete slab foundation, the proposed dwelling would need to be offset from the top of the steep fill embankment by approximately 4m to achieve the required FOS >1.5 through the building platform.

Under adverse groundwater conditions and assuming a concrete slab foundation , the proposed dwelling would need to be offset from the top of the steep fill embankment by approximately 7m to achieve the required FOS > 1.2 through the building platform

Alternatively, a palisade wall has been modelled along the top of the steep fill embankment. 250mm SED timber piles at 0.75m centres have been used for the purpose of the slide model. Piles extend a minimum of 5m below the existing ground level pin into the assumed underlying weathered deposits. With the use of a palisade WHANGARE! DIS wall, a FOS >1.5 and FOS >1.2 is achievable under normal and adverse groundwater conditions.

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8.3.3. Summary

A summary of the Factors of Safety through the building property for the existing and finished surfaces under normal and adverse ground conditions is tabulated below:

Groundwater	Factor of Safety (FOS)	
	Existing Surface	Finished Surface
Normal groundwater	1.1	>1.5
Adverse groundwater	0.6	>1.2

Table 5. Summary of slope stability analysis results

The analysis confirms that a minimum Factor of Safety of 1.5 is available under normal groundwater conditions and 1.2 under adverse conditions provided the house is either setback from the top of the steep fill embankment by 7m, or a palisade wall is constructed along to top of the steep embankment. Results of the slope analysis indicate that installation of drainage and maintenance of stormwater systems is critical at this site.

9. FOUNDATIONS AND RETAINING

Many of the soils located within the Northland region are considered to be expansive soils. There are three basic types of soil naturally occurring in the Northland Area: sand, silt and clay. Clay soils are generally classified as "expansive." This means that a given amount of clay will tend to expand (increase in volume) as it absorbs water and it will shrink (lessen in volume) as water is drawn away. The action of seasonal shrink/swell of soils can have a significant impact on foundations of structures and also on other components of developments such as services, claddings, windows, doors, pavements etc. It is evident from historical reports and site inspections that the effect of expansive soils is a major problem in Northland.

It is considered that the building site does not meet the requirements for Good Ground as defined in the New Zealand Building Code. Foundations will require design in accordance with AS2870 for Class M moderately expansive soils or a specific engineered design for expansive and plastic soils. This will require design by a Chartered Professional Engineer with relevant geotechnical experience.

For a concrete slab foundation, it is recommended the slab is of stiffened raft or waffle raft design to help mitigate the effects of expansive soils. This will need to be underpinned with piles to extend through the uncontrolled fill at the front (southern side) of the building platform.

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For a suspended timber floor, piles will also need to extend through the uncontrolled fill at the front (southern side) of the building platform.

It is expected that there is uncontrolled fill of up to 2 - 3m deep on the southern side of the property. Inspection will be required at the time of pile excavation to ensure foundations extend through the fill and pin into natural ground beneath.

The proposed dwelling will need to be offset 7m from the top of the steep fill embankment along the southern property boundary. Alternatively, a palisade wall will be required. This should be constructed along the top of the steep fill embankment, consisting of 250mm short end diameter (SED) timber piles at 0.75m centres. extended a minimum of 5m below the existing ground level. These will need to be bored and concreted rather than driven, to avoid disturbance to close neighbouring properties. The proposed dwelling can then be constructed up to the palisade wall.

An alternative option is to place the rainwater and stormwater attenuation tanks on the southern side of the building platform, near the southern lot boundary. The uncontrolled fill across the building platform could be removed and re-compacted at the same time as excavation for the tanks. Inspection will be required to ensure all uncontrolled fill is removed and suitably re-compacted in accordance with NZS4431. , has been coulded to be required. *

RETAINING

RETAINING The tanks will act as a form of in-ground retaining, removing the need for a palisade wall. The proposed dwelling will then be set-back from the top edge of the steep fill embankment by at least the diameter of the tanks. Piling will not be required if the uncontrolled fill is removed and suitably re-compacted.

The existing Retaining Wall 2, located along the western property boundary, has been designed for a light surcharge of 5kPa. Where building loads are calculated to be greater than 5kPa, a setback distance from Retaining Wall 2 will be required.

FOUNDATIONS AND RETAINING

10. CONCLUSIONS AND RECOMMENDATIONS

Geotechnical investigations indicate that the proposed building site is presently unstable, however subsoil properties have adequate strength parameters necessary for the proposed development. Development will need to be carried out in accordance with proper engineering practice and the following recommendations:

- Foundations will require specific engineering design to accommodate the effects of expansive soils. The site soils are considered to be moderately expansive and classified as Class M in accordance with AS2870.
- For a concrete slab foundation, it is recommended the slab is of waffle raft or stiffened raft design. This will need to be underpinned with piles to extend through the uncontrolled fill at the front (southern side) of the building platform.
- For a suspended timber floor, piles will also need to extend through the uncontrolled fill at the front (southern side) of the building platform.
- 4. It is expected that there is uncontrolled fill of up to 2 3m deep on the southern side of the property. Inspection will be required at the time of pile excavation to ensure foundations extend through the fill and pin into natural ground beneath.
- 5. The proposed dwelling will need to be offset 7m from the top of the steep fill embankment along the southern property boundary. Alternatively, a palisade wall will be required. This should be constructed along the top of the steep fill embankment, consisting of 250mm short end diameter (SED) timber piles at 0.75m centres, extended a minimum of 5m below the existing ground level. These will need to be bored and concreted rather than driven, to avoid disturbance to close neighbouring properties. The proposed dwelling can then be constructed up to the palisade wall.
- 6. An alternative option is to place the rainwater and stormwater attenuation tanks on the southern side of the building platform, near the southern let boundary. The uncontrolled fill across the building platform could be removed and re-compacted at the same time as excavation for the tanks. Inspection will be required to ensure all uncontrolled fill is removed and suitably recompacted in accordance with NZS4431. The tanks will act as a form of inground retaining, removing the need for a palisade wall. The proposed dwelling will then be set-back from the top edge of the steep fill embankment by at least the diameter of the tanks. Plling will not be required if the uncontrolled fill is removed and suitably re-compacted.

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- 7. The existing Retaining Wall 2, located along the western property boundary. has been designed for a light surcharge of 5kPa. Where building loads are calculated to be greater than 5kPa, a setback distance from Retaining Wall 2 will be required.
- 8. Any excess fill material shall be disposed of off-site.
- 9. All stormwater from the property shall be collected and piped to the Council stormwater system within Ngunguru Road.
- 10. Specific design of stormwater attenuation may be required once the layout and final configuration of the development is established and can be completed as part of the Building Consent application.
- 11. To ensure the integrity of Retaining Wall 1 (situated along the northern property boundary) is maintained the stormwater tanks must be offset at least the height of the wall; this would be 3m from Retaining Wall 1. Alternatively, specific design of the tanks may reduce the offset required.

Providing that the above-mentioned recommendations are followed then the conclusion drawn from the site investigation and analysis of the property as identified above, is that the site is capable of developed as proposed, and in terms of Section 71 and 72 of the Building Act 2004, it can be confirmed that:

- The land on which the building work is to take place is not likely to be subject to subsidence or slippage,
- The building work itself is not likely to accelerate or worsen or result in ji. subsidence or slippage of that land or any other property.

result in

RECOMMENDATIONS ARELL DISTRICT

RECOMMENDATIONS ARE RECOMME

11. LIMITATIONS

This report has been prepared for the benefit of Phillip Stubbs as our client with respect to geotechnical investigation for residential development and for Whangarei District Council approval of the proposal as defined in the brief. It shall not be relied upon for any other purpose. The reliance by other parties on the information or opinions contained in this report shall, without our prior review and agreement in writing, be at such parties' sole risk.

Opinions and judgments expressed herein are based on our understanding and interpretation of current regulatory standards, and should not be construed as legal opinions. Where opinions or judgments are to be relied on they should be independently verified with appropriate legal advice. Any recommendations, opinions, or guidance provided by Cook Costello in this report are limited to technical engineering requirements and are not made under the Financial Advisers Act 2008.

Recommendations and opinions in this report are based on data from hand augered boreholes with shear vane measurements and Scala penetrometer testing undertaken on site. The nature and continuity of subsoil conditions away from the boreholes and Scalas are inferred and it must be appreciated that actual conditions could vary considerably from the assumed model.

During excavation and construction the site should be examined by an Engineer or Engineering Geologist competent to judge whether the exposed subsoils are compatible with the inferred conditions on which the report has been based. It is possible that the nature of the exposed subsoils may require further investigation and the modification of the design based on this report. In any event it is essential that the firm is notified if there is any variation in subsoil conditions from those described in the report as it may affect the design parameters recommended in the report.

Cook Costello have performed the services for this project in accordance with the standard agreement for consulting services and current professional standards for environmental site assessment. No guarantees are either expressed or implied.

There is no investigation which is thorough enough to preclude the presence of materials at the site which presently, or in the future, may be considered hazardous. Because regulatory evaluation criteria are constantly changing, concentrations of contaminants present and considered to be acceptable now may in the future become subject to different regulatory standards which cause them to become unacceptable and require further remediation for this site to be suitable for the existing or proposed land use activities.

S George

Engineering Technician BSc, NZDE G Harding

NHANGA

Chartered Professional Engineer CPEng, IntPE(NZ), BE, BSc, MIPENZ CILBOA



Code Compliance Certificate BC1700484

Section 95, Building Act 2004

Issued: 12 March 2018

The Building

Street address of building: 1830 Ngunguru Road

Whangarei 0173

Legal description of land where building is located: LOT 2 DP 361651

LLP: 116427

Building name: N/A

Location of building within site/block number: N/A

Level unit number: N/A

Current, lawfully established use: Detached Dwelling

Year first constructed: 2017

The Owner

K A Stubbs P S Stubbs

1830 Ngunguru Road

RD₃

Whangarei 0173

Phone number: N/A

Mobile number: 021992499

Facsimile number: N/A
Email address: N/A
Website: N/A

First point of contact for communications with the building consent authority:

Contact Person

A1 Homes Northland

PO Box 183 Ruakaka 0151

Phone number: 4330200

Mobile number: 021729724

Facsimile number: N/A

Email address: mark.russell@a1homes.co.nz

Website: N/A

Street address/registered office: 1830 Ngunguru Road

Whangarei 0173





Building Work

New Dwelling

Building Consent Number:

BC1700484

Issued by:

Whangarei District Council

Code Compliance

The building consent authority named below is satisfied, on reasonable grounds, that -

(a) The building work complies with the building consent.

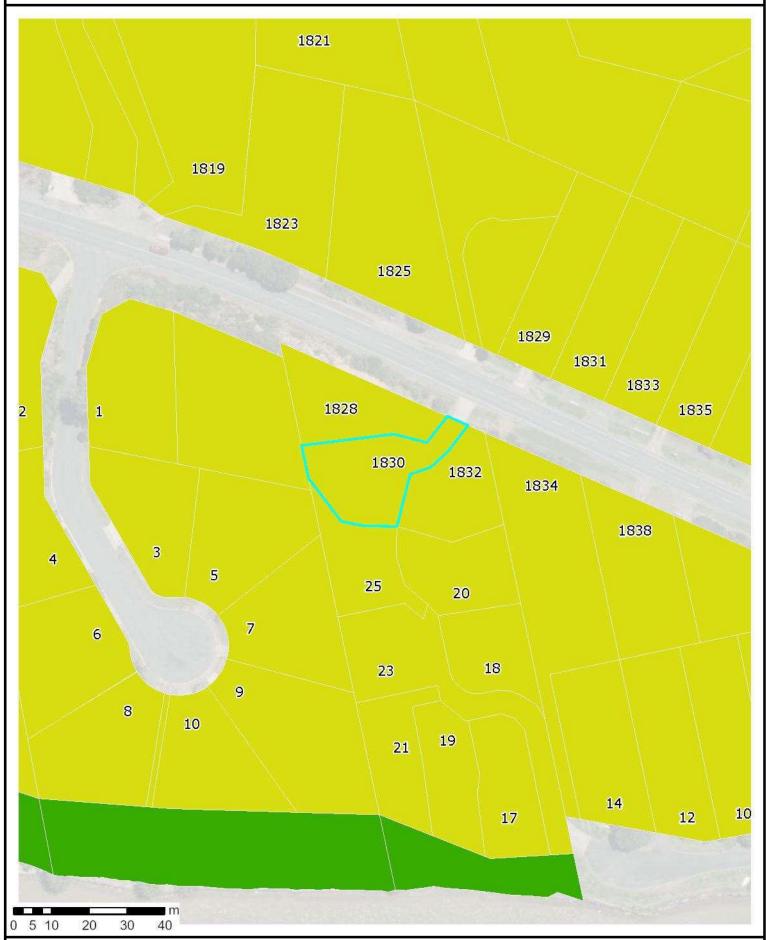
Kylee Mangu

Support Assistant – Building Processing On behalf of Whangarei District Council 12 March 2018

Date

Operative District Plan - Area Specific Matters





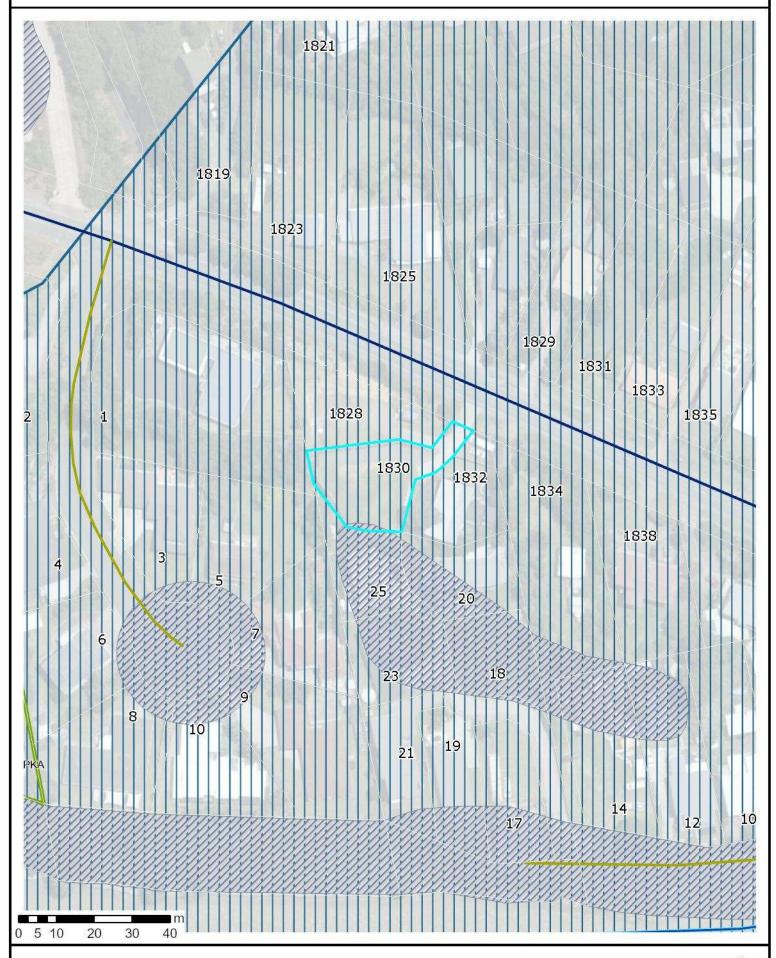
The information displayed is schematic only and serves as a guide. It has been compiled from Whangarei District Council records and is made available in good faith but its accuracy or completeness is not guaranteed.

20 November 2023 Scale 1:1,000



Operative District Plan - District-Wide Matters





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20 November 2023 Scale 1:1,000



Operative District Plan - Map Legend



District-Wide Matters

Area Specific Matters Multi Title Site Industrial Zones Energy, Infrastructure and **Historical and Cultural** Transport **Values** Designation Light Industrial Zone Notable Tree Overlay Airport Runway Precinct Heavy Industrial ---- Indicative Road Heritage Item Overlay Development Area Zone - National Road Heritage Area Overlay Sites of Significance Residential Zones Regional Road to Maori - Arterial Road Open Space and Large Lot Areas of Significance Recreation Zones Residential Zone **Primary Collector** to Maori Road Low Density Natural Open Papakāinga Residential Zone Secondary Collector Space Zone Road General Residential Open Space Zone **Natural Environment** Access Road Zone Values Sport and Active Low Volume Road Medium Density Recreation Zone Residential Zone Esplanade Priority Strategic Road Area Protection Area Coastal Marine Area Rural Zones Strategic Railway (CMA) boundary Protection Line Special Purpose Zones Settlement Zone Goat Control Areas Rescue Helicopter Residential Sub-Flight Path **QRA Quarrying** ! Airport Zone Resource Area Zone National Grid Tower Hospital Zone Settlement Zone **QRA Mining Area** Northpower Tower Centre Sub-Zone Port Zone CEL-Cat1 QRA Buffer Area Settlement Zone Ruakaka Equine Industry Sub-Zone National Grid Line Zone QRA 500m Indicative Setback Northpower Overhead Rural Production Critical Line Cel-Cat1 Zone **Outstanding Natural** Feature Northpower Critical Rural Lifestyle Zone Overhead Lines CEL Outstanding Natural Future Urban Zone Landscape Northpower Critical Strategic Rural Underground Lines CEL Industries Zone General District-Wide Fonterra Kauri Milk Processing SRIZ -Ancillary Irrigation Hazards and Risks ----- Air Noise Boundary Farms Outer Control Coastal Erosion Boundary Hazard 1 Helicopter Hovering Commercial and Mixed Coastal Erosion Hazard 2 Noise Control Flood Susceptible Local Centre Zone **Boundary Overlay** Areas Neighbourhood Rail noise alert area Mining Hazard Area 1 Centre Zone Rail vibration alert Mining Hazard Area 2 Commercial Zone area Mining Hazard Area 3 Mixed Use Zone Coastal Environment Overlay Town Centre Zone Outstanding Natural City Centre Zone Character Area Waterfront Zone

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Shopping Centre

Zone

Parcel Information is sourced from the Land Information New Zealand (LINZ) Data Service.

High Natural Character Area